EVALUATING REUSE OF SLUDGE FROM WATER TREATMENT FOR WASTEWATER TREATMENT

AN ASSESSMENT OF EFFECTS OF SPENT ALUM SLUDGES ON WASTEWATER TREATMENT IN GOTHENBURG, SWEDEN

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ABSTRACT

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LIST OF SYMBOLS AND ABBREVIATIONS

Symbols

Ļ	Formation of solid precipitate separated from solution.
\rightarrow	General purpose reaction arrow (action).
4	Equilibrium arrow (reversible process).
(s)	Denotes a solid state.

Abbreviations

DWTP	Drinking water treatment plant		
WWTP	Wastewater treatment plant		
ТОС	Total organic carbon		
СЕРТ	Chemically enhanced primary treatment		
COD	Chemical oxygen demand		
SS	Suspended solids		
Р	Phosphorus (mg P/l)		
o-P	Orthophosphate (mg P/l)		
PO ₄ -3	Phosphate ion (mg PO_4^{-3}/l)		
IC	Ion chromatograph		
ICP-MS	Inductively coupled plasma mass spectrometer		
NOM	Natural organic matters		
EPA	Environmental Protection Agency		
Al	Aluminium		
WWF	Wet weather flow		
SVI	Sludge volume index		
AL	Alelyckan (drinking water treatment plant)		
LA	Lackarebäck (drinking water treatment plant)		
AL sludge	Alum drinking water sludge from the Alelyckan		
	drinking water treatment plant		
LA-1 sludge	Alum drinking water sludge from the Lackarebäck		
	drinking water treatment plant, settling tank 1		
LA-5 sludge	Alum drinking water sludge from the Lackarebäck		
	drinking water treatment plant, settling tank 5		
rpm	Revolutions per minute		
SEK (kr)	Swedish Krona. All conversions to/from GBP (£)		
	based on the May 2014 average exchange rate of 1 kr		
	$= \pm 0.09.$		

Referencing

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1. INTRODUCTION

1.1 Overview and problem statement

The concept of sustainability is one which engineers globally need to grasp to ensure the needs of the present are met without compromising the ability of future generations to meet their own needs [1]. This is particularly relevant for water and wastewater engineering where arguably the effects of engineering decisions are observed more rapidly and clearly than in other disciplines.

In Gothenburg, Sweden's second largest city situated on its west coast, the disposal of drinking water treatment plant (DWTP) sludge via the Rya wastewater treatment plant (WWTP) was subject to evaluation in terms of both economic and environmental sustainability. While treating and supplying drinking water for approximately 500,000 inhabitants of Gothenburg [2] [3], several DWTPs generate alum sludge, a by-product of raw water treatment to produce drinking water. The disposal of alum DWTP sludge via the partly combined sewer system through Gothenburg to the Rya WWTP was alleged to be costly and detrimental from a sustainability standpoint [4] [5] [6], such that an evaluation of the detrimental and beneficial effects was desirable.

Previous studies [7] [8] [9] suggested the effects of alum DWTP sludge were limited to physical or primary treatment in the WWTP, with little effect on biological or secondary treatment processes. Consequently, this project focussed particularly, although not exclusively, on evaluating the effects of DWTP sludge on primary treatment, including removal of phosphorus, organics and solids (primary and secondary settling), as well as assessing the handling (thickening and dewatering) of DWTP sludge. Primarily of interest were the chemical, economic and environmental effects of disposing DWTP sludge via the Rya WWTP, and whether any benefits outweighed the drawbacks.

Previous research, although existing globally, was limited in its relevance, with conclusions varying case by case [10]. Upon the conclusion of this 4 month project, a pragmatic solution was sought, such that in-depth chemical analysis was excluded to the benefit of facilitating a project practically relevant to the Rya WWTP.

Founded on a literature review, practical insight in Gothenburg and supported by simple bench-scale jar testing, this project contributed towards a city-wide economic and environmental evaluation of disposing alum DWTP sludge in Gothenburg.

1.2 Aim and objectives

1.2.1 Overall aim

As part of a collaboration between Chalmers University of Technology and the companies running the drinking wate did wastewater treatment plan did between the chemical, economic and environmental effects of the reuse of two alum DWTP sludges in wastewater treatment at the Rya WWTP were evaluated.

1.2.2 Individual research objectives

Evaluating the reuse of alum DWTP sludges at the Rya WWTP was achieved by means of both theoretical and practical studies:

- Current understanding of alum DWTP sludge effects on primary wastewater treatment for phosphorus removal were evaluated through review of existing literature.
- Probable and confirmed effects of handling alum DWTP sludge on major treatment operations at the Rya WWTP were evaluated by means of a brief literature review from case studies nationally and globally.
- Chemical characterisation of two alum DWTP sludges from the Alelyckan and Lackarebäck DWTPs enabled a comparison between the two sludges.
- Wastewater samples from the Tankgatan sampling station in Gothenburg representative of the influent municipal wastewater at the Rya WWTP were also chemically characterised.
- The effects of Alelyckan and Lackarebäck DWTP sludges on primary wastewater treatment were analysed through bench-scale jar testing with municipal wastewater. Effects on total phosphorus, organics content and solids (primary and secondary settling) among other effluent quality parameters were evaluated, both chemically and economically.
- The handling (thickening and dewatering) characteristics of Alelyckan and Lackarebäck DWTP sludges and their influence on the most appropriate sludge processing method was evaluated through experimentation and review of existing literature.
- Upon conclusion of the above research objectives, an appropriate proposal for an economically and environmentally sustainable solution to the management of DWTP sludge in Gothenburg was suggested.

1.3 Value of research

Completion of the aforementioned research objectives benefitted all those directly involved in the project by contributing data and subsequent conclusions towards an economically and environmentally sustainable solution to the management of DWTP sludge in Gothenburg. In combination with a consultant completing a wider review, results of this project were valuable for the companies running the drinking water and wastewater treatment plants in Gothenburg and the municipal government.

1.3.1 Environmental value

The adoption of sustainable environmental policies since the late 1980s transformed Sweden into a role model in water management [11]. Accordingly, DWTP sludge handling in Gothenburg should be sustainable, despite the fact that legislation surrounding disposal of DWTP sludges is deficient globally. As an inescapable byproduct of drinking water production, DWTP sludge production is unlikely to decrease in the foreseeable future [12] such that sustainable DWTP sludge management is a proactive aim with high environmental value.

1.3.2 Economic value

Kretslopp och vatten, Göteborg Stad (Department of sustainable waste and water, Gothenburg City) and Gryaab AB (operator of the Rya WWTP) believed the current situation was not sustainable from an economic standpoint, based on previous reports and rough estimations of handling DWTP sludge at the Rya WWTP [6] [7] [8] [13] [14] [15]. Reduced efficiencies in several major treatment operations were inferred, of which biogas production and legislation for WWTP sludge reuse and disposal in Sweden were highlighted as primary concerns for future operating and investment costs.

1.3.3 Limitations

Throughout the conduct of the project, attempts were made to ensure practicality and relevance to the Rya WWTP in Gothenburg. Experimental design used fresh DWTP sludges and wastewater, collected from the Alelyckan and Lackarebäck DWTPs and the Tankgatan wastewater sampling station.

The 4 month time-scale of this project required a balance between sufficient review of literature and ensuring practicality of experimental design. This project focussed experimental design on the chemical and economic effects of DWTP sludge on primary wastewater treatment, while broader aspects of the evaluation, including more

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substantial economic analysis and potential sludge re-uses, were analysed in a separate project by a fellow student, in addition to a consultant for the city of Gothenburg.

Experimentation was carried out in Spring 2014, and wastewater and sludge samples were understandably subject to natural time-based and weather-based variations. Appropriate procedures were taken to minimise errors through duplication of tests, maintenance of consistent experimental environments and accurate modelling of actual WWTP conditions.

1.4 Structure of report

1.4.1 Project planning

A theoretical review was considered in section 2.1, focussing on phosphorus species in wastewater and current understanding of the mechanisms of their removal in a WWTP. Additionally, throughout the background study and literature review, attention was given to other parameters which were expected to be affected by DWTP sludge, specifically organics content and solids (primary and secondary settling).

In section 2.2, details of practical insights and specificities of the DWTPs and Rya WWTP in Gothenburg were followed by consideration of handling (thickening and dewatering) alum DWTP sludge. Subsequently, drawing on valuable experience from other case studies and literature, probable and confirmed effects of alum DWTP sludge on major treatment operations at the Rya WWTP were evaluated.

Finally in section 2.3, a literature and methodological review of previous approaches, significant variables, and key conclusions from earlier research with alum DWTP sludge in phosphorus removal was conducted to ensure practically relevant experimental design.

2. BACKGROUND STUDY AND LITERATURE REVIEW

2.1 Phosphorus removal from wastewater

Phosphorus speciation in wastewater is briefly evaluated, followed by consideration of the methods of phosphorus removal in primary wastewater treatment at a WWTP. Subsequently, the behaviours of aluminium and alum DWTP sludge in wastewater, and mechanisms of phosphorus removal by alum DWTP sludge and comparable mineral precipitants (salts) are reviewed.

2.1.1 Phosphorus and phosphate speciation

In wastewater, phosphorus occurs as various species of phosphate [16], specifically orthophosphate (o-P), polyphosphates (also referred to as complex or condensed phosphates) such as pyrophosphate, tripolyphosphate and trimetaphosphate [17], and particulate organic phosphates tied up in organic matter [17] [18].

P speciation varies depending on proportions of commercial, industrial or municipal components of wastewater as well as the collection system design and the time of travel before the wastewater reaches the WWTP. Jenkins et al. [19] proposed approximate concentrations of phosphate species in typical raw domestic sewage as shown in Table 2.1 below. A revision 20 years later by the same author [20] saw the constituent proportions relatively unchanged.

	Concentration			
Phosphate form	$(mg P l^{-1})$	$(\mathbf{mg} \mathbf{P} \mathbf{l}^{-1})$		
	(Jenkins et al. (1971) [19])	(Jenkins & Hermanowicz (1991) [20])		
Total phosphorus	≲10	≲ 7		
Orthophosphates	5	3-4		
Tripolyphosphates	3	2.2 (tripoly and pure together)		
Pyrophosphates	1	2-3 (urpory- and pyro- together)		
Organic phosphates	≲1	1		

Table 2.1 Approximate concentrations of phosphate species in a typical raw domestic sewage Jenkins et al. [19] and several others [21] [22] [23] [24] concluded that o-P represents the dominant form of P in raw municipal wastewater, typically with an o-P/total P fraction of between 0.6 and 0.9 [25]. While ultimately the measurement and removal of total P is most important for wastewater treatment, the o-P form is the most biologically available and easiest to precipitate [26].

2.1.2 Wastewater treatment plant

P removal from wastewater occurs at a wastewater treatment plant (WWTP), where the influent wastewater is treated via a series of unit operations to restore the wastewater to a given standard before discharge to a receiving water body [27].

2.1.2.1 Phosphorus removal

Generally P removal can be part of primary, secondary, or tertiary treatment processes depending on the WWTP configuration [28]. With reference to the simplified schematic of the Rya WWTP in Fig. 2.1 on the following page, primary (or physical) treatment removes the majority of solids via bar screens and subsequently settling tanks, where primary sludge settles and can be removed [29]. Secondary (or biological) treatment makes use of forcefully mixing wastewater with bacteria and oxygen to digest pollutants, and can be tailored specifically, such as for nitrogen (N) removal at the Rya WWTP [29]. Tertiary (or advanced) treatment removes dissolved substances, namely colour, metals and nutrients through a physical, chemical or biological process, such as microscreens in the form of discfilters to remove P at the Rya WWTP [30] [31].

This project focussed on chemical addition to the raw wastewater before primary treatment, termed chemically enhanced primary treatment (CEPT), or pre-precipitation [22], modelling the discharge of alum DWTP sludge from the DWTPs through the partly combined sewer system to the Rya WWTP.

2.1.2.2 Pre-precipitation and co-precipitation

With pre-precipitation, the formed precipitates are removed together with the primary sludge, and the wastewater continues to secondary (biological) treatment, as shown in Fig. 2.1 below [17]. Commonly used chemicals in pre-precipitation include a variety of aluminium compounds, of which hydrated aluminium sulphate (alum) is the most common [22] [32]. However, the full potential of some chemicals for phosphorus removal may not be realised solely in primary treatment, such that phosphorus removal is a combination of chemical and biological treatment [33], termed co-precipitation [34]. This describes the use of iron salts, such as ferrous sulphate in the Rya WWTP, which is used with a non-nitrifying activated sludge system with co-precipitation of phosphorus [35] [36]. The formed precipitates are removed either from primary settling together with the primary sludge or from secondary settling with waste biological sludge, as shown in the schematic of Fig. 2.1 below [17].



Fig. 2.1 Simplified schematic of pre-precipitation (CEPT) and co-precipitation chemical influence at the Rya WWTP, adapted from Isaias et al. [37], Barr [18] and Gryaab AB [38]

2.1.2.3 Pre-precipitation effects on other parameters

With pre-precipitation, the addition of chemicals often has a multitude of effects apart from solely phosphorus precipitation. Commonly, the metal (aluminium (Al), and iron (Fe)) salts used for chemical P removal can be also used for enhanced particulate, colloidal BOD₅ (5-day biochemical oxygen demand) and SS (suspended solids) removal [22]. As a result, truly soluble, particulate, colloidal and condensed phosphate species can be removed via precipitation, coagulation, flocculation, adsorption and sedimentation during pre-precipitation.

Takács et al. [39] among many others [18] [12] [40] [41] [32] [42] concluded that while chemical phosphorus removal with metal salts is widely used and relatively straightforward to design, complex precipitant and/or adsorption reactions and mechanisms of P binding to metal complexes is not fully understood. Barr [18] highlighted that several factors, both known and unknown, may affect the efficiency of P removal, and the situation is more complex for wastewater, which is heterogeneous by nature, such that reactions are often incomplete and numerous side reactions can occur.

Nonetheless, at present a variety of metal salts can be employed at a number of different locations in the process stream in a WWTP. While ferrous sulphate, $(FeSO_4 \cdot 7H_2O)$ in its most common commercial form [43]), is used at the Rya WWTP, a commonly used Al salt in phosphorus removal is aluminium sulphate (alum), $(Al_2(SO_4)_3 \cdot 14H_2O)$ in its commercially used form [22]) [27].

2.1.3 Aluminium sulphate and alum DWTP sludge

Alum is also valuable as a flocculating agent in the coagulation and flocculation treatment steps in DWTPs to remove turbidity, colours, humic substances, tastes and odours [44] [45]. Coagulation involves the addition of coagulant chemicals to form larger particles [46], while flocculation, aided by gentle mixing, is the gathering together of particles in the water, forming flocs, which settle and can be removed as sludge in a settling step, such as a sedimentation basin [46].

As shown in the simplified schematic of Fig. 2.2 below, the Alelyckan and Lackarebäck DWTPs operate comparable processes, with four major physical-chemical steps of chemical coagulation and flocculation with aluminium sulphate, sedimentation, and filtration and adsorption, as well as the chemical operations of pH adjustment and disinfection (chlorination) [2] [6] [59] [60].



Fig. 2.2 Simplified schematic of Alelyckan and Lackarebäck DWTPs, adapted from van der Schraft [47] Studies of DWTPs in Canada using alum as a coagulant [48] confirmed that more than 99.98% of the total (dosed) aluminium left the plant in the form of sludge, such that only a small fraction of Al remains in drinking wat a colloidal or dissolved form. Consequently, it can be assumed with negligible error that all the dosed aluminium leaves the DWTP as sludge and enters the sewer system. While disposal via the sewer system is a marked improvement from earlier practices of discharging to the nearest water body [12], it may be environmentally unsustainable, in contrast to the environmental aims of Gothenburg [49]. During wet weather, the partly combined sewer system in Gothenburg relies on overflows into the River Göta to prevent excess water overloading the sewer system [4]. While DWTP sludges are currently classified as non-hazardous in the European List of Wastes [50] (code number 19 09 02) with no specific legislation concerned with DWTP sludge disposal [12], several studies [51] [52] suggested that several forms of aluminium, of which alum DWTP sludge contains high levels [53], are toxic to aquatic life. However, in a literature review Babatunde & Zhao [12] noted there exists limited research, such that the magnitude of potential toxicity effects due to DWTP sludge and its metals contents and their relevance to the cirumneutral pH typical of water bodies require further investigation.

Kim et al. [54] determined alum DWTP sludge contained significant amounts of aluminium hydroxide, aluminium organic complexes and water-soluble aluminium, and suggested, along with several others [53] [55] [54] [56] [57] [58] [45], alum DWTP sludge can remove phosphorus as various species of phosphate by means of its aluminium components.

2.1.4 Phosphorus removal by alum DWTP sludge

2.1.4.1 Aluminium and alum in water

The fate and behaviour of aluminium, and hence also aluminium salts, in aqueous solutions is complex [48]. Hossain & Bache [59] concluded that when aluminium salts were released into water, such as in the flocculation and coagulation processes in a DWTP, most of the aluminium hydrolyses to form aluminium hydroxides, as detailed in studies by Driscoll and Schecher [60], Howells et al. [61] and Sullivan & Singley [62]. Kim et al. [54] reported the amorphous form of aluminium (probably low soluble Al(OH)₃(s) [63]) represented over 75% of the reactive aluminium in spent alum sludge from DWTPs, congruent with Georgantas et al. [45] and several others [43] [64] [65] [66] concluding that spent alum sludge from DWTPs is predominantly amorphous aluminium hydroxide. Consequently, for alum DWTP sludge, mechanisms of P removal by aluminium hydroxides are dominant.

2.1.4.2 Mechanisms of phosphorus removal by aluminium hydroxides

Guan et al. [67] and many others [68] [69] [70] [71] [72] [73] [74] [75] proposed that phosphates (including orthophosphates, polyphosphates and tripolyphosphates) are adsorbed onto metal (hydr)oxides by forming monodentate, bidentate or binuclear complexes through a ligand exchange mechanism. The ligand exchange mechanism, a reaction with hydrous oxides, involves the phosphate ions (in solution) replacing OH groups on the surface of the hydroxylated oxides (on the alum sludge surface), such that the OH groups and the phosphate ions undergo a substitution [69] [76] [77] [78], as suggested by Rajan [79] in a proposed exchange reaction with HPO_4^{-2} in Fig. 2.3 below.



Fig. 2.3 Adsorption of HPO₄⁻² by ligand exchange, adapted from Rajan [79]

Sims & Ellis [80] and others [68] [70] studied further intricacies of the ligand exchange mechanism. Proposed reactions from Rajan [79], and more recently Yang et al. [68] and Kamiyango et al. [70], were substantiated by Georgantas & Grigoropoulou [69] whereupon experiments showed that the uptake of phosphates was associated with an increase of pH in the solution, as would be expected with the ligand exchange mechanism releasing OH^- ions in the solution [68] [76] [77] [80] [81].

However Georgantas & Grigoropoulou [69] and Yang et al. [68] noted that the actual reaction is expected to be yet more complex. Reddy & DeLaune [77] also proposed that organic anions also compete strongly with phosphate for ligand exchange sites.

2.1.5 Phosphorus removal by metal salts

When traditional metal salts such as alum and ferrous sulphate are used for P removal, chemical precipitation is expected to be the dominating mechanism [21] [17] [41].

2.1.5.1 Aluminium sulphate

Several authors [21] [19] [82] [83] stressed that the required alum dose in wastewater treatment is larger than the theoretical stoichiometric precipitation reaction of P with alum (molar ratio (Al:o-P) of 1:1) due to reactions with other constituents of wastewater, such as organics, or side reactions in the form of hydroxides [84]. Accordingly, Guyer [83] and others [18] [35] reported actual case studies required a higher molar ratio (Al:o-P) of 2:1, while design manuals [26] recommend molar ratios (Al:o-P) of between 1.72:1 and 2.3:1 for P removal of between 85% and 95% from municipal wastewater.

2.1.5.2 Ferrous sulphate

Many uncertainties surrounding P removal with alum are applicable to other mineral precipitants, including ferrous sulphate [28]. However, knowledge of the dose of iron sulphate at the Rya WWTP, and the cost of purchasing (560 kr/tonne, 2013 [85]) would enable an economic evaluation of the effectiveness of alum DWTP sludge in P removal compared to current precipitation chemicals. Based on operational data from the Rya

WWTP for 2012 and 2013, the current weight ratio of iron sulphate (Quickfloc S, 17.6% Fe²⁺ [85]) to total P was determined as 7.6:1, as shown in Table 2.2 below.

Total P influent* (mg/l) [86]	3.4
Total P before tertiary treatment (discfilters) (mg/l) [30] [87]	0.3
Total P removal by iron sulphate** (mg/l)	3.1
Total wastewater pumped to plant, 2013 (Mm ³) [88]	119.5
Total P removal by iron sulphate, 2013 (tonnes)	370.5
Iron sulphate dosed, 2013 (tonnes) [88]	2822
Weight ratio, iron sulphate (Quickfloc S, 17.6% Fe ²⁺):total P	7.6:1

*Rya WWTP influent modelled by samples from Tankgatan sampling station, October 2012 data [86] **Total P removal in primary and secondary treatment attributed entirely to iron sulphate addition

Table 2.2 Calculation of weight ratio, iron sulphate (Quickfloc S, 17.6% Fe²⁺):total P

2.2 Practical insight in Gothenburg

A practical overview of the system of DWTPs and the Rya WWTP in Gothenburg is followed by consideration of handling (thickening and dewatering) alum DWTP sludge.

Secondly, while the effects of alum DWTP sludges on total P, organics content and solids (primary and secondary settling) in primary treatment were analysed experimentally, probable and confirmed effects of handling alum DWTP sludge on other major treatment operations at the Rya WWTP are evaluated via a brief literature review, drawing on past research from the Rya WWTP and other studies case studies nationally and globally.

Finally, previous assessments, relevant future proposals and rough economic evaluations of the effects of alum DWTP sludge on the Rya WWTP are considered.

2.2.1 Drinking water treatment plants

As shown in as shown in Fig. 2.4 opposite, the southern branch of the Göta älv (River Göta), flowing southwest through the west coast of Sweden, serves as a major raw water intake for the drinking water supply at Lärjeholm, to the north-west of Gothenburg.



DWTP	Total solids from DWTP sludge (tonnes)		
Lackarebäck	509		
Alelyckan	895		
Lerum	36		
Mölndal	160		
Härryda	24		
Total	1624		
Table 2 3. Total solids from			

Table 2.3: Total solids from DWTP sludge received at Rya WWTP, 2012, from Tumlin [6]

Fig. 2.4 The River Göta catchment, with catchment boundary, locations of WWTPs, PSPs and raw water intake, adapted from Lindhe et al. [3]

Of the seven municipalities which own the Rya WWTP, DWTP sludge is received at the Rya WWTP from Ale (included in Gothenburg), Härryda, Lerum and Mölndal [6]. As shown in Table 2.3, the Alelyckan and Lackarebäck DWTPs accounted for over 85% of total solids from DWTP sludge received at the Rya WWTP in 2012. Consequently, since the contribution from Härryda, Lerum and Mölndal was relatively small, this project focussed on Alelyckan and Lackarebäck.

2.2.1.1 Alelyckan and Lackarebäck system description

Alelyckan DWTP, to the north-west of Gothenburg, and Lackarebäck DWTP, to the south-west, are each responsible for about half of an (average) 168,000 m³ of drinking water produced each day for the city [3] [89]. While both DWTPs use a similar treatment process with aluminium sulphate as a coagulant, the raw water intake for the Alelyckan and Lackarebäck DWTPs can differ, resulting in fluctuating qualities and quantities of DWTP sludge [90]. Shown schematically in Fig. 2.5, a unique system of tunnels and reservoirs is used [90].



Fig. 2.5 Schematic of water supply in Gothenburg, adapted from Nilsson [91]

From the Lärjeholm raw water intake water can be pumped to the Alelyckan DWTP or to the Delsjöarna Lakes (Lake Lilla Delsjön and Lake Stora Delsjön), which along with Lake Rådasjön, act as reservoirs for both DWTPs with a residence time of approximately four months [90]. The residence time is not necessarily the time taken for water pumped from the raw water intake to reach the Lackarebäck DWTP, which could be a few weeks under unfavourable conditions [91]. With monitoring stations upstream on the River Göta, the Lärjeholm intake can also be closed, due to heavy rainfall [3], construction activities near the river [90], saltwater intrusion [3] or accidental discharges or spills of wastewater or chemicals [3], such that both DWTPs receive water only from the Delsjöarna Lakes, for a maximum of 3-4 weeks [3] [90] [89]. The Lärjeholm intake is closed approximately 100 days per year [3].

Mendel [90] reported high amounts of colour, turbidity and bacteria in the River Göta, while water from the Delsjöarna Lakes carried significantly reduced pollutant loads. Although water quality in the River Göta is generally high, with the majority of parameters within the old Swedish national guidelines ('gamla kungörelsen') given by SLVFS 1993:35 [90], water quality in the Delsjöarna Lakes is substantially better [90], and this is expected to be reflected in the contents of the DWTP sludges.

The residence time in the Delsjöarna Lakes aids the settling of suspended sediments and microorganisms, resulting in lower total solids as DWTP sludge, as validated in Table 2.4. Not only does the Alelyckan DWTP generate

significantly greater total solids than

DWTP	Total solids from DWTP sludge (tonnes)			
	2009	2011	2012	
Lackarebäck	429	545	509	
Alelyckan	721	798	895	
Total	1150	1343	1404	

Table 2.4 Total solids from Alelyckan and Lackarebäck DWTP sludge received at Rya WWTP in 2009, 2011 and 2012, from Tumlin [6]

Lackarebäck (an average of over 60% more annually for 2009, 2011 and 2012), but also total solids from all DWTP sludge has increased annually, with an average annual increase for 2011 and 2012 of approximately 85 tonnes, an issue discussed further in later sections with regards to current management at the Rya WWTP.

From Table 2.3, the total solids received at the Rya WWTP could be crudely averaged to approximately 4.45 tonnes per day. However, a continuous discharge of sludge is unlikely due to the storage and flushing mechanisms employed at both Alelyckan and Lackarebäck. At the Alelyckan DWTP 1 of 4 settling tanks is flushed weekly, such that the DWTP sludge may have settled for up to 4 weeks before the bulk of the sludge is flushed into the sewer [92], and while at the Lackarebäck DWTP, the DWTP sludge is flushed more regularly, such that the sludge from Alelyckan is 'older', sludge from Lackarebäck is held in storage tanks such that neither DWTP sludge from Alelyckan or Lackarebäck is 'fresh' upon reaching the Rya WWTP [93]. Such fluctuations could pose a challenge for handling DWTP sludge at the Rya WWTP.

2.2.2 Rya wastewater treatment plant

As the sole WWTP in Gothenburg, the Rya WWTP treated wastewater from approximately 918,000 population equivalents (including industry and based on water consumption) in 2013 [88]. Wastewater flows through a partly combined sewer system with transport time at dry weather flow between 16 and 20 hours [38].

2.2.2.1 Wastewater treatment

Wastewater flows up to 10 m³/s receive full biological/chemical treatment. As explicated in Fig. 2.1 earlier, this involves screening, iron sulphate addition, primary

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sedimentation, activated sludge with pre-denitrification, secondary sedimentation, postnitrification in trickling filters, post-denitrification and finally disc filters, after which treated water is released into the mouth of the River Göta [38] [94]. As explicated by Mattsson et al. [36], for peak wet weather flows (WWF) greater than 10 m³/s, or when the capacity of secondary treatment is reduced, excess wastewater can be treated without further biological treatment via temporary on-demand direct precipitation in existing primary clarifiers [95], with an average use of 55 days per year between 2006 and 2010 [36]. Such a scenario would not only dilute the DWTP sludge, but also the aforementioned combined sewer overflows may reduce the quantity of DWTP sludge in the influent wastewater.

2.2.2.2 Sludge handling

As well as expected effects on primary treatment, alum DWTP sludges were expected to influence the handling (thickening and dewatering) characteristics of sludge, and consequently influence the most appropriate sludge processing method.

At the Rya WWTP, primary and secondary sludge undergoes belt gravity thickening (with polymer addition) before it enters a digester, along with fats, oils, grease (FOG) and food waste from local restaurants and commercial kitchens [96] [97] [94]. Over 65 GWh of biogas from anaerobic digestion, almost twice the energy consumption of the Rya WWTP [88], was sold and upgraded in 2013 [98] [38]. Finally, the sludge is dewatered via centrifugation (with further polymer addition) for composting as a soil enhancer (or disposal) [38] as shown in the schematic of Fig. 2.6 below.



Fig. 2.6 Simplified schematic of sludge processing/disposal at the Rya WWTP, adapted from Gryaab AB [38] Sludge thickening and dewatering

Sludge thickening reduces the volume of the sludge for subsequent processing steps [28], although the product retains the properties of a liquid [99]. Common thickening processes include filtration, which can be mechanical, gravity or vacuum based [100]. Sludge dewatering uses mechanical dewatering equipment, such as centrifuges (typically at 1800-3500 rpm [101]), to remove more water and produce material with the properties of a solid [28] [99].

Water distribution and removal in sludge

Sludges can be visualised as consisting of flocs, acting hydrodynamically as single particles [84], where water exists in sludge as identifiable forms, such as in the classification by Vesilind & Martel [102] shown in Fig. 2.7 below.





Free water is not bound to the particles and surrounds the sludge flocs, and can be removed by simple gravity or vacuum filtration [102] [103]. Interstitial water is held within the floc structure and can be removed when the floc is broken up by mechanical dewatering devices such as centrifuges [102] [104]. Surface water and bound water can only be released by thermochemical destruction of the particles [102] [105] [106]. Sludge dewatering and thickening processes focus on removal of free and interstitial water, which compose the majority of the water in sludge [106].

Alum DWTP sludge handling

Power et al. [107] noted that alum DWTP sludge can be difficult to thicken and dewater. Wang et al. [64] suggested this behaviour is due to the high affinity of aluminium hydroxide for water, while Pan et al. [108] attributed poor dewaterability to organic materials in the source raw water (at the DWTP) producing smaller flocs with a higher water content.

At WWTPs, Wilson et al. [109] described high concentrations of alum DWTP sludge producing a WWTP sludge that was "difficult to handle", while Öman [7] concluded

that not only was dewatering deteriorated, but polymer consumption (and hence cost) increased by 11%. Sludge dewatering methods often rely heavily on polymer addition (sludge conditioning) to achieve desirably high percentages of dry solids by weight (in the range 18 to 25% wt. DS. for alum DWTP sludge [110]) [111]. Investigation into appropriate conditioning of alum DWTP sludge was beyond the scope of this project, and has been studied in detail by Wu et al. [111] and Zhao & Bache [110]. However, analysis of the water distribution in the Alelyckan and Lackarebäck sludges enabled a basic evaluation of an appropriate sludge processing method.

2.2.2.3 Further effects of DWTP sludge

Experience and conclusions were attained from comparable treatment processes at the Bromma and Himmerfjärden WWTPs in Stockholm county, Sweden, where pilot-scale testing on handling alum DWTP sludge was carried out [8] [112] [113]. Of note were the effects on influent wastewater, biological treatment, biogas production and heavy metals concentrations in WWTP sludge.

Influent characteristics

Starberg [14] reported a general increase in incoming chemical oxygen demand (COD) while incoming phosphorus content remained unchanged. This corresponds with results from Salotto et al. [9], and estimates from the Rya WWTP which suggested less than 0.2% P in the WWTP sludge originated from the DWTP sludge, as shown in Table 2.7 below. Increases in COD could be attributed to a high organics content of the DWTP sludge, with the Bromma WWTP noting approximately 50% aluminium hydroxide and 50% organics content [7].

An increase of sludge at the primary sludge outlet in was noted at the Bromma WWTP, [7]. Salotto et al. [9] determined the majority of DWTP sludge settled out with the primary sludge in the WWTP, and following an aluminium mass balance at the Bromma WWTP, a similar conclusion was reached [7]. This increase of sludge, estimated as 10% of total solids at the Rya WWTP in 2012 as shown in Table 2.5 on the following page, is expected to have effects on existing sludge handling equipment as noted by Wiechers [114]. Operational experience from the Bromma WWTP noted that bar screens required more maintenance, pump efficiency was reduced in pipelines upstream of primary treatment, along with higher probabilities of pipes clogging when the DWTP sludge had a significant solids content [7].

Parameter		Estimated amount in DWTP sludge, 2012* [kg/year]	Estimated amount in Rya WWTP sludge, 2012 [kg/year]	DWTP sludge share of total amount, 2012 [%]	
Solids	Total Solids	1624000	15982000	10	
Metals	Zn	113	11783	1.0	
	Cd	0.2	15	1.3	
	Pb	19	486	3.9	
Phosphorus	Total P	730	471000	0.2	

* Alelyckan (2009) and Lackarebäck (2012) DWTPs

Table 2.5 Estimated pollutant load in DWTP and WWTP sludge for 2012, from Tumlin [6]

Biological treatment

As noted earlier, the effects of DWTP sludge were expected to be limited to primary treatment [7] [8] [9], coinciding with no effect noted on biological treatment processes at the Bromma WWTP [8], and no significant benefit or detriment noted by Culp & Wilson [115]. Effects on biological treatment for nitrogen (N) removal, although expected to be minimal, were beyond the scope of this study, although extensive work was done by Salotto et al. [9] and Hsu & Pipes [116] on the effects of alum DWTP sludge and aluminium hydroxide on the activated sludge process and secondary settling of activated sludge.

Anaerobic digestion and production of biogas

At the Bromma WWTP, biogas production did not increase corresponding to the increased organic material fed to the anaerobic digesters with DWTP sludge [7]. Starberg [8] contended this was due to inert chemical solids from the DWTP sludge taking up space in the reactor (rather than directly inhibiting the digestion process), a view shared by Boyke & Rupke [117] and several others [7] [9] [118]. A reduction in biogas production could also be due to a reduction in residence time in the digesters due to increased sludge flow, as noted by Öman [7].

Heavy metals and REVAQ certification

The heavy metals of cadmium, zinc and lead were identified by Tumlin [6] as "priority metals" for the Rya WWTP for the proposed limits on metals concentrations in WWTP sludge introduced by the Swedish EPA [7]. Considering Table 2.5, while the DWTP sludge accounted for 10% of total solids in 2012, it accounted for proportionally less cadmium, zinc and lead. Furthermore, Table 2.6 on the following page suggests a dilution effect of the DWTP sludge, to the extent that without the addition of the DWTP sludge the Rya WWTP sludge would exceed proposed limits from the Swedish EPA on zinc concentrations for 2015.

Parameter		Average in Rya WWTP sludge including DWTP sludge, 2012 [mg/kg TS]	Average in Rya WWTP sludge excluding DWTP sludge, 2012 [mg/kg TS]	Proposed maximum for 2015 [mg/kg TS]	Proposed maximum for 2023 [mg/kg TS]
Metals	Zn	770	813	800	750
	Cd	0.95	1.0	1.0	0.9
	Pb	32	33	35	30
	Ni	19	19	40	35

Table 2.6 Effect of DWTP sludge on pollutant load in WWTP sludge for 2012, from Tumlin [6]

Similar conclusions were reached at the Bromma WWTP, where the addition of DWTP sludge resulted in approximately 20% reduction in metals concentrations in WWTP sludge (lead, cadmium, cobalt, chromium, copper, mercury, manganese, and zinc)

2.2.3 Economic analysis of alum DWTP sludge effects

Primary concerns in an economic evaluation of the effects of alum DWTP sludge on the Rya WWTP were future quantities of DWTP sludge (affecting operating costs) and the installation of sludge hygienisation at the Rya WWTP (affecting investment costs).

2.2.3.1 Quantity of DWTP sludge

The future quantity of DWTP sludge is highly dependent on the operations of the DWTPs and the dose of aluminium sulphate as a flocculant. Tumlin [6] contended that future decreases in raw water quality from the River Göta could result in higher doses of flocculant at the Alelyckan DWTP and increased DWTP sludge generation. Conversely, at the time of this study, an ultrafiltration plant was under construction at the Lackarebäck DWTP [119]. When in operation, this membrane filtration system was expected to reduce flocculant dose requirements at the Lackarebäck DWTP, such that DWTP sludge generation was decreased [6]. Despite the annual increases in total solids from DWTP sludge detailed in Table 2.4, Kretslopp och vatten, Göteborg Stad believe future quantities of DWTP sludge will be unchanged [6].

2.2.3.2 Hygienisation of sludge at the Rya WWTP

Hygienisation processes, such as thermophilic digestion or thermal hydrolysis [120] [121], are designed to kill harmful microorganisms before reusing sludge in for agriculture or for making soil by REVAQ certification (Ren Växtnäring från Avlopp – pure nutrients from sewage) [122]. In 2012 less than 6% of total sludge from the Rya WWTP went to agriculture [38], and options for improving the hygienisation of WWTP sludge are under consideration for increasing REVAQ sludge quantities [122]. While development of a hygenisation process at the Rya WWTP was still in the early planning stages at the time of writing this report, it was believed such an investment would be increased by handling the DWTP sludge [6] [122].

2.2.3.3 Operating and investment Costs

Operating and investment costs of handling alum DWTP sludge at the Rya WWTP were rough economic estimations obtained from previous assessments and future proposals. Operating costs are summarised in Table 2.7 below. The foremost investment cost surrounded the development of the hygienisation processes, expected to cost in the region of SEK 100-200 million [6]. Rough estimates from Tumlin [6] suggested handling DWTP sludge increased investment costs by SEK 10-20 million depending on the specific hygienisation process and its location in the treatment process [122].

DWTP sludge	DWTP sludge Justification of cost			
effect		year)		
Operating costs of sludge handling*	DWTP sludge estimated as 10% of total sludge (based on total solids). Cost is 10% of electricity use, polymer and water consumption for treating sludge, transport and disposal cost. Includes 10% income from biogas sales.	SEK 2.76 million		
Lost income from biogas production	Lost income from equivalent quantity of sewage sludge, assuming no biogas production by DWTP sludge.	SEK 1 million		
Nitrogen removal from DWTP sludge supernatant	Highly dependent on future cost of nitrogen removal [6].	SEK 0.2 – 1 million		
Total costs of handlin	SEK 3.96 – 4.76 million (≈ £356,400 - £428,400)			

*Excluding staff costs and potential increased wear in equipment, due to lack of conclusive data N.B. All costs based on 2011 financial data and analysis detailed by Tumlin [6]

Table 2.7 Operating costs of handling alum DWTP sludge at the Rya WWTP

It was expected, should the alum DWTP sludge be useful for P removal, that the DWTP sludge offered savings in the use of ferrous sulphate as a precipitation chemical at the Rya WWTP. Attempts to quantify this saving based on experimental data, and comparison to the above estimated operating costs, formed the basis of the economic evaluation of the effects of alum DWTP sludge at the Rya WWTP.

2.3 Literature and methodological Review

An overview of chemical characterisation of alum DWTP sludges from recent literature is followed by a review of experimental handling of alum DWTP sludge samples and applicability to ensuring a practically relevant project.

Conclusions from literature evaluating alum DWTP sludge effects on total P removal, organics content and solids (primary and secondary settling) among other effluent quality parameters are summarised.

Finally, consideration of the factors affecting P removal with alum DWTP sludge is presented while detailing limitations and assumptions for a pragmatic experimental design. Attention is also given to the challenge of accurately measuring total P.

2.3.1 Chemical composition of alum DWTP sludge

With alum typically dosed at a rate of 10 to 150 mg/l (or approximately 1.58 to 23.66 mg/l as Al) as a coagulant in DWTPs [123], it was expected that the Al component of alum DWTP sludge dominated its chemical composition. Babatunde et al. [72] noted that the Al component could be 1-5 orders of magnitude greater than all other elements.

DWTP sludge could also contain substantial proportions of inorganic suspended solids and organic matter. Inorganic suspended solids includes mineral solids, of which iron and calcium typically dominate in alum DWTP sludge, while organic matter includes humic substances, plankton, algae, and bacteria [14]. Mineral solids such as iron and calcium were expected to have a beneficial effect on P removal, due to common use as metal salts for P removal [84], while Ghassemi & Recht [124] suggested organics could reduce phosphate precipitation efficiency through formation of soluble organic-metal complexes (as proposed in Fig. 2.8 below) rather than metal-phosphate precipitates.



Fig. 2.8 Alum coagulation of humic substances, adapted from Rebhun and Lurie [125] The summary of chemical compositions of alum DWTP sludges from literature in Table 2.8 on page ____ confirmed the dominance of the Al and organics (often analysed as total organic carbon (TOC)) components. The aforementioned "priority metals" of cadmium, zinc and lead are also included in Table 2.8.

The summary in Table 2.8 also highlighted the variability of both proportional and absolute concentrations of components, attributed to different raw water and treatment processes of DWTPs [68], but also different treatment (handling) of the alum sludge during experimentation. Throughout this project, attempts were made to ensure the handling of the alum sludge during analysis resulted in practically relevant experimentation, as detailed in the review of previous experimental approaches (section 2.3.2) and the limitations and focus on a pragmatic experimental design (section 2.3.3).

2.3.2 Review of previous experimental approaches

Relevant studies with alum DWTP sludge and its effects on primary wastewater treatment have been carried out globally, with notable previous studies from Ireland [53] [72] [126] [127] [55] [128], Greece [45] [69] [129], Canada [130] [18] [127] [131] [132], China [56] [133], Korea [54], Australia [134] and Iraq [58]. Evaluation of previous research and the various experimental approaches adopted was required to ensure practically relevant experimentation.

2.3.2.1 Experimental handling of alum DWTP sludge

The gelatinous metal hydroxide content and the aforementioned affinity for water was reported to make experimental handling of alum DWTP sludge challenging [18] [131] [130]. While the moisture content of a collected DWTP sludge sample was dependent on the dewatering processes (if any) employed at the DWTP, of more interest were the different approaches to treating sludge during experimentation.

Sludge collected as dewatered (moisture content 72-75% [55] [53] [128]) was commonly ground, sieved and analysed. Without dewatering, Georgantas & Grigoropoulou [69] and Georgantas et al. [45] used the sludge 'as collected' from the bottom of a sedimentation tank at the DWTP. By far the most common approaches to handling the sludge were air drying [53] [134] [54] [72] [133] (moisture content approximately 10% [53]) or oven drying [58] [57] [56] (no moisture) and grinding to a powder which could be handled in a dry or re-suspended form [57].

Parameter Unit* -		U	Sample number (see below)													
		1	2	3	4	5		6	7	8	9	1	0	11	12	
Solids	% solids	%	-	25.6	56.2	23.0	53.	8	-	-	-	-		-	69.9	57.8
	Al as Al_2O_3	mg/g	110	151	177	170	13	7 4	61	43	297	38-125	15-	300	-	-
	Fe as Fe_2O_3	mg/g	17	8.6	5.7	6.7	16.	6 1	2.1	3.3	102	16.3-26.	1 5-	66	-	-
Motolo	Ca as CaO	mg/g	7.2	-	-	4.0	-	1	1.7	0.82	29	4.5-54.6	3-	50	-	56
Wietais	Zn	mg/kg	-	20.8	17.2	65.0	14.	2	-	30.0	33.9	1.7-50		-	150	79
	Cd	mg/kg	0.3	< 1.10	< 0.49	0.51	< 0	50	-	-	0.50	-		-	1.10	< 0.30
	Pb	mg/kg	<1	3.6	3.0	19.0	7.5	5	-	5.0	44.0	5-32		-	43.0	6.0
Phosphor	rus Total P	g/kg	0.99	-	-	0.58	-			0.12	3.50	0.5-4.4	0.2	-4.4	0.80	0.58
NOM	TOC	g/kg	78	-	-	173	-	97 98		-	-	8.5-225		-	-	
pН	pH	-	6.8-7.1	7.2	5.9		6.7	7 - 7.0 -		-	5.0	-8.2	-	-		
* /kg or /	* /kg or /g is per kg or g of DWTP sludge; air dried, dewatered or untreated as detailed below - not analysed															
Sample	Locatio	n	Date	Reference	Treatm	ent	Sample	e Location				Date	Refe	erence	Treatment	
1.	Loveland, Colora	ado, USA	2011	[71]	Air dried	l	7.	Kildare, Ireland				2009	[72]		Dewatered	
2.	Charlotte, Florida	, USA	2001	[135]	None		8.	Literature review from Babatunde & Zhao			-	[12]		-		
3.	Desoto, Florida, I	JSA	2001	[135]	None		9.	9. Literature review from DeWolfe		-	[136	5]	-			
4.	Kildare, Ireland		2014	[126]	Dewater	ed	10 Literature review from Makris & O'Connor		-	[137]	-				
5.	Sarasota, Florida,	USA	2001	[135]	None		11.Alelyckan DWTP, Gothenburg1993			[13]		None				
6.	Dublin, Ireland		2006	[53]	Dewater	ed	12. Alelyckan DWTP, Gothenburg 2009 [13] No			None						

Table 2.8 Major chemical composition of alum DWTP sludge from literature

		Concentration						
Parameter	Unit	Tankgatan, 2006/2007*	Tankgatan, October 2012	Tankgatan, November 2012**				
Suspended solids	mg/l	294	200	170				
NOM (as TOC)	mg/l	40	-	-				
Total N	mg/l	14	24	16				
Total P	mg/l	1.7	3.4	1.8				
*substantial dilution noted due to unusually high precipitation [86] - not analysed								

*substantial dilution noted due to unusually high precipitation [86] **dilution noted due to high precipitation throughout November 2012 [86]



	Unit	Concentration				
Parameter		'Weak'	'Medium'	'Strong'		
Total solids	mg/l	300	700	1250		
Suspended solids	mg/l	70	200	450		
NOM (as TOC)	mg/l	80	150	280		
Total N	mg/l	20	50	90		
Total P	mg/l	5	10	18		
Alkalinity (as $CaCO_3$)	mg/l	50	100	160		

Table 2.10 Classification of municipal wastewater I	by major chemical
composition from Sincero & Sincero	[138]

2.3.2.2 Sludge processing

Without dewatering, alum DWTP sludge was expected to have a solids concentration of around 1% if automatically removed from the sedimentation tank, or 2% if manually removed [139]. While considerable literature exists on treatment and disposal of alum DWTP sludge, Droste [140] and several design manuals [141] [142] stress that every sludge should be considered separately due to significant site-specific influences. Nonetheless, mechanical dewatering, including vacuum filters, filter presses and centrifuges, has been reported to achieve up to 20% solids [143], while alternative techniques reported achieving higher solids concentrations, including sludge freezing (25-30%) and drying beds (30-50%) [144]. Numerous plant-based studies [145] [146] [147] have concluded centrifugation to be practically (and economically) effective for alum DWTP sludge processing.

2.3.2.3 P removal rates and alum DWTP sludge doses

Pilot-scale testing at the Bromma WWTP, showed an increase in P reduction in primary treatment from 21% to 32% with an average of 433 kg Al/day as DWTP sludge [7]. However, few laboratory studies focussed on a practically relevant setup, with the majority modelling wastewater [53] [55] [54] [56] [57] [58] [45], commonly as KH₂PO₄ [69] [45] [55] [128] [57], focussing analysis on particular phosphate species, specifically o-P [58] [57] [134] [53], condensed phosphates [133] [55] and organic phosphates [54], or modelling the alum sludge as aluminium hydroxide [118] [116].

However, Georgantas & Grigoropoulou [69] completed a practically applicable study based on jar testing, with alum DWTP sludge collected from a primary sedimentation tank and dosed, untreated, into municipal wastewaters without pH control, summarised in Table 2.11 below.

Study	Molar ratio (Al:PO ₄ - ³)	% Total P removal (%)			
	0.68:1	11%			
Coorgontos & Crigoronoviou (1) [60]	1.35:1	18%			
Georganias & Ongoropoulou (1) [69]	2.04:1	31%			
	3.40:1	46%			
	0.96:1	18%			
Coorgontos & Crigoronoviou (2) [60]	1.90:1	37%			
Georganias & Ongoropoulou (2) [69]	2.87:1	50%			
	4.78:1	64%			

 Table 2.11 Total P removal from wastewater in jar testing with spent alum DWTP sludge from literature

 Total P measurement

Thermal acidic persulphate digestion was traditionally used for in the analysis of total P [148], converting the P into o-P, which could be measured by ion chromatography [129] [149], or more commonly via a colorimetric or spectrophotometric method using appropriate reagents [53] [129] [55] [133] [45].

2.3.2.4 Effects of alum DWTP sludge addition on other effluent quality parameters

Effects of alum DWTP sludge on organics content and solids (primary and secondary settling) among other effluent quality parameters including total N and pH were also evaluated.

Organics, total N and pH

Organics

Dosing untreated alum DWTP sludge into municipal wastewater and measuring organics as COD, Guan et al. [150] reported a maximum of over 15% enhanced COD removal with 75 mg Al/l sludge addition, compared to controls without sludge addition (28 to 38% COD removal). However, dosing 75 mg Al/l required a huge and impractical volume of sludge [150]. After maximum COD removal, decreases in removal could be attributed to significant organic matter being released by the sludge. Georgantas & Grigoropoulou [69] also reported enhanced COD removal (approximately 40% removal at a molar ratio (Al:PO4⁻³) 4.78:1), while in a plant-based study, Nelson et al. [151] reported no changes in efficiency of COD removal. It was concluded that organics removal depended on the individual characteristics of the sludge, determining whether organics removed by the sludge exceeded the organics released [150].

Total N

Barr [18] evaluated total Kjeldahl nitrogen (organic nitrogen, ammonia (NH₃), and ammonium (NH₄⁺)) and ammonia when dosing alum and alum DWTP sludge to municipal wastewater, and concluded insignificant effects on either. Due to low levels of oxidation in primary treatment, nitrate and nitrite concentrations in the influent were negligible, such that no effect on the nitrate and nitrite forms was detectable either. An earlier study by Hsu & Pipes [116], modelling alum DWTP sludge as aluminium hydroxide, reached similar conclusions, with doses of up to 300 mg Al/l having no effect on ammonia, organic nitrogen, nitrite or nitrate. The aforementioned ligand exchange mechanism suggested pH increases with uptake of phosphates [69]. However, pH variation depends on the alkalinity of the wastewater, or 'buffering capacity' to neutralise acids. The carbonate system and ammonia contribute to the buffer characteristics [152], such that wastewater with alkalinity (as $CaCO_3$) above 80 mg/l could prevent significant quick changes in pH [153].

With the majority of studies modelling wastewater [53] [55] [54] [56] [57] [58] [45], and thus lacking the 'buffering capacity', pH control was required and had a significant effect on P removal [53] [55], influencing the solubility of Al with an optimum P removal around pH 5 to 6 [54] [45] [129]. When dosing alum DWTP sludge to actual municipal wastewater, Huang & Chiswell [134] and Georgantas et al. [45] noted very small increases in pH, citing this natural buffering capacity of wastewater.

Primary settling

Typical measurements of primary settling efficiency were suspended solids (SS) (filterable through 0.45 µm filter paper) in the settled wastewater. Past studies on settling were rare and somewhat inconclusive. Hsu & Pipes [118] modelled alum DWTP sludge by dosing aluminium hydroxide to municipal wastewater and concluded less than 30 mg Al/l as sludge would increase SS. However, with further Al dose SS decreased and it was noted that large doses (above 150 mg Al/l) of aluminium hydroxide in municipal wastewater altered settling mechanisms from 'flocculant settling' to 'zone settling', as described by Fitch [154]. Conversely, Guan et al. [150] did not report any increases in SS when alum sludge was added, reporting only increased removal efficiencies, with a maximum of 90% SS removal with 30 mg Al/l compared to controls without sludge addition (52-66%).

Secondary settling

While several studies [7] [9] confirmed the majority of DWTP sludge settled out with the primary sludge in the WWTP, consideration of alum DWTP sludge effects on secondary settling with activated sludge was included to attempt to clarify the limited conclusions on primary settling. Unfortunately however, past literature was similarly inconclusive. Hsu & Pipes [116] modelled adding aluminium hydroxide to various biological treatment processes, and concluded aluminium hydroxide addition increased the settling velocity of activated sludge from approximately 3.75 cm/min (control, no DWTP sludge) to a maximum of 4.75 cm/min (20 mg Al/l), after which settling velocity

decreased with increased aluminium hydroxide addition. Conversely, a plant-based study with alum DWTP sludge by Camp, Dresser & McKee, Inc. [155] reported only decreased efficiency of secondary settling, consequently requiring either increased secondary settling capacity or addition of polymers to aid settling.

2.3.3 Limitations and focus on a pragmatic experimental design

In addition to the aforementioned pH control, several other experimental design factors could influence P removal with alum DWTP sludge. Limitations of experimental design were defined to ensure practicality and relevance to the Rya WWTP. Jar testing would model primary wastewater treatment at the Rya WWTP with alum DWTP sludge dosed as a precipitation chemical for P removal.

Mixing and settling time and energy

While Lijklema [156] and Gillberg et al. [157] concluded orthophosphate adsorption onto aluminium was significantly influenced by mixing speed, several recent jar-testing studies with alum DWTP sludge [18] [69] [45] [150] modelled the mixing and settling with three distinct mixing periods of rapid mixing, slow mixing and quiescent settling. In this study, selection of the mixing and settling time and energy was based on the approximate 1 hr 40 min retention time for wastewater in primary treatment at the Rya WWTP [38] and recent jar-testing studies with alum DWTP sludge from literature.

pН

The influence of pH on P removal was dependent on the buffering capacity of the wastewater. pH was not controlled in the jar tests as it detracted from practically modelling the Rya WWTP, where pH adjustment would be economically unviable.

Temperature

The temperature of wastewater during primary treatment at the Rya WWTP was approximately 10 °C [158], while jar testing was carried out at room temperature (approximately 20 °C). Hossain [132] reported inconsistent responses of alum DWTP sludge capacity for P adsorption from wastewater when temperature was varied from 5 to 19 °C. It was therefore assumed that potential effects of temperature variation were insignificant for this study. More of concern in previous literature were the effects of storage conditions on the capacity of alum DWTP sludge to remove P.

Storage and aging of alum DWTP sludge

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After 2 months aging at 25 °C, Georgantas et al. [45] reported a 20% decrease in efficiency of alum DWTP sludge in P removal from synthetic wastewater. Conversely, Yang et al. [128] reported marginal effects of aging on P removal from synthetic wastewater, even after 18 months of storage at room temperature (20 °C). Potential effects of aging were believed to be attributable to mineralisation of aluminium and structural transformation of aluminium hydroxide [128] [80] [159], and were beyond the scope of this study. Sludge samples collected from DWTPs and stored at approximately 10 to 15°C were used for approximately 2 months after collection in this study, assuming negligible changes of the alum DWTP sludge characteristics.

Wastewater and alum DWTP sludge samples

The use of synthetic wastewater or modelling alum DWTP sludge with aluminium hydroxide in previous studies did not produce a practically relevant model. This study used actual samples of municipal wastewater and alum DWTP sludge, which although may have introduced uncertainty due to heterogeneities, were sought to model the practical situation at the Rya WWTP accurately.

Alum DWTP sludges collected from DWTPs were handled 'as collected' from the DWTPs as much as possible throughout experimentation. Common approaches from past literature of air drying or oven drying prior to experimental handling were deemed detrimental to ensuring this study was practically relevant, as no sludge treatment (thickening or dewatering) took place at the Alelyckan or Lackarebäck DWTPs.

Finally, to obtain a wastewater sample not 'contaminated' by the regular release of DWTP sludge into the sewer system, influent from the Tankgatan sampling station was considered and assumed to be a representative sample of the influent at the Rya WWTP, with past characterisation detailed in **Table 2.9** on pg ____ alongside a classification of municipal wastewater by Sincero & Sincero [138] in **Table 2.10**.

Metal ion dose

The molar ratio (metal: PO_4^{-3}) was commonly used for dose of precipitation chemicals [84] [40]. Previous studies with alum DWTP sludge dosed in municipal wastewater reported a range of ratios (Al: PO_4^{-3}) dosed into wastewater, from 0.68 to 4.78 [69]. An approximation of the molar ratio for the current scenario at the Rya WWTP, based on 2012 data is shown in Table 2.12 below.

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Total P received at Rya WWTP, 2012 (tonnes) [88]	451
Assumed o-P/total P fraction*	0.75
Total Al dosed at DWTPs, 2012** (tonnes) [15]	215
Molar ratio Al:PO ₄ ⁻³	0.73

*Based on typical o-P/total P fraction for municipal wastewater of 0.6 to 0.9 [25] **Based on data from Alelyckan and Lackarebäck DWTPs, assuming all dosed Al is received at Rya WWTP

Table 2.12 Estimation of molar ratio (Al:PO₄⁻³) for current situation at Rya WWTP With the molar ratio (Al:PO₄⁻³) of the current scenario approximated as low relative to past studies, dose of DWTP sludge would be based on modelling the current scenario and higher molar ratios (Al:PO₄⁻³) of 2:1 and 3:1. In addition to facilitating comparison with literature for DWTP sludge, Barr [18] and O'Blenis & Warriner [131] used a similar molar ratio to compare the capacity of DWTP sludge for P removal with alum, concluding alum DWTP sludge was between 5 and 10 times less effective than alum (as Al). A comparison with alum was deemed more relevant than with the current precipitant of ferrous sulphate in the Rya WWTP due to the latter removing P more effectively in secondary unit operations than primary treatment [17] [22] [33] [84] [124].

Total P measurement

With the aforementioned ligand exchange mechanism, several species of P were removed by alum DWTP sludge, such that measurement of total P was most relevant experimentally (and also practically for wastewater treatment).

Ion chromatography is one of the most commonly used methods of analysis of anions and cations in water and wastewater [160], offering quick, safe and highly sensitive analysis [161] [162]. However, Ruiz-Calero & Galceran [163] and several others [164] [165] expressed concern at the interference of high concentrations of some ions causing false readings of other ions, particularly the high concentrations of sulphate produced with thermal acidic persulphate digestion and its interference with measuring phosphate. Consequently, following thermal acidic persulphate digestion to convert P into o-P, in this study total P was measured using both ion chromatography and a colorimetric method with reagents to determine whether the use of expensive and toxic reagents in the colorimetric method could be replaced with the quicker and cleaner alternative of ion chromatography.

3. METHODS AND MATERIALS

Details of the collection and storage of DWTP sludge and wastewater samples (section 3.1) is followed by a summary of the analysis methodologies for characterisation of the DWTP sludge and wastewater samples (section 3.2 to 3.4). Since the majority of analyses were carried out to standard methods, only the aforementioned total P methodologies (ion chromatography and colorimetric method) and metals analysis are described comprehensively.

Finally the bench-scale jar testing (section 3.5), further solids (settling) with activated sludge (section 3.6) and DWTP sludge handling (section 3.7) setups are outlined.

3.1 Sample collection and storage

All sample collection locations referred to throughout this chapter are shown on the map in Fig. B.2 in Appendix B. All analyses were carried out at the Environmental Chemistry Laboratory at Chalmers University of Technology.

3.1.1 DWTP sludges

Alum DWTP sludge samples were collected from the Alelyckan and Lackarebäck DWTPs on the morning of 20th March 2014. Sludge was manually scraped/collected from the bottom of one of four sedimentation tanks at Alelyckan DWTP (denoted AL), as shown in a) and b) of Fig. B.1 in Appendix B prior to the flushing of the tank. At the Lackarebäck DWTP, sludge was automatically removed from the eight sedimentation tanks into storage tanks. Sludge samples were collected from two storage tanks (denoted LA-1 and LA-5) via a tap as shown in c) of Fig. B.1 in Appendix B. AL and LA DWTP sludges were stored 'as collected' in 1 and 5 litre HDPE plastic containers respectively. The containers were kept in basement storage with temperatures approximately 10 to 15 °C and were used for approximately 2 months after collection.

3.1.2 Wastewater samples

Wastewater samples were collected on the mornings of 18th March 2014 and 16th May 2014 from the Tankgatan sampling station by Fredrik Davidsson, Environmental Engineer at Gryaab AB. The Tankgatan sampling station in the north of Gothenburg, as shown on the map in Fig. B.2 in Appendix B, was not part of a combined sewer system [166]. Overnight weather preceding the collection of the first sample was dry, whereas the second sample was collected after a period of heavy rainfall.

Wastewater samples were stored in 10 litre HDPE plastic containers in basement storage (with the DWTP sludge samples) with temperatures approximately 10 to 15 °C. Testing and characterisation with the first wastewater sample (25 litres) was completed within 20 days of collection, while experimentation with the second sample (10 litres) was completed within 10 days of collection.

3.1.3 Activated sludge sample

A sample of activated sludge was collected for analysis of DWTP sludge effects on secondary settling. The activated sludge sample was collected ahead of secondary settling tanks at the Rya WWTP, as shown in d) and e) of Fig. B.1 in Appendix B, and stored in 10 litre HDPE plastic containers. Samples were collected on the morning of 8th May 2014 after a period of heavy rainfall, and testing and analysis was completed in the laboratory within 6 hours of collection.

3.2 Analysis methodologies

A summary of analysis methodologies for all analysed parameters is shown in Table 3.1 on pages ___. Due to the page limit for this thesis, a detailed description of all methodology is not feasible. Accordingly reference has been made to the standard methods followed.

Appropriate procedures were taken to minimise errors through duplication of tests (as detailed in Table 3.1), maintenance of consistent experimental environments in the laboratory (room temperature 20 °C) and thorough inverting/shaking of samples after removing from storage. All containers of the same 'batch' of collected sludge or wastewater were assumed to have the same characteristics. Samples, particularly after jar testing, were analysed as soon as possible after completion of the test. Otherwise samples were preserved overnight by refrigeration at 4 °C.

3.2.1 Digestion for total P measurement

Digestion for total P measurement in wastewater used peroxodisulphate/alkali (Oxisolv oxidizing decomposition reagent, Merck). 15 ml of unfiltered wastewater sample and 350 mg Oxisolv were added to a digestion vial. A duplicate wastewater sample was also used as a control. Lids were secured and the digestion vials heated in a CEM MARS 5 Microwave oven at 100 psi (≈ 689 kPa) for 30 minutes to raise the temperature over 120 °C according to the microwave manual. When heated over 120 °C, nitrogen containing compounds were oxidized to nitrate by peroxodisulphate and the release of acid

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lowering the solution to pH < 2 enabled the oxidation of P-containing compounds to o-P.

3.2.2 Ion chromatography

Ion chromatography measures concentrations of ions by separating them based on their interaction with a resin (stationary phase) and eluent (mobile phase) [167]. These phases differ between an anion column, which attracts anions (including phosphate, sulphate, nitrate and nitrite), and a cation column, which attracts cations (including ammonium and calcium) [168]. Ionic species move through the columns at different rates, and the retention time of different species, measured at outlet by conductivity, determines the concentrations of ionic species in the sample [167] [169]. Conductivity is plotted against time to produce a chromatogram with the concentration of each ion correlated to the height and the breadth of the [168].

Digested wastewater samples were filtered through a 0.45 μ m syringe filter and diluted 10 × to ensure the high sulphate concentration (from peroxodisulphate/alkali digestion) was within detection limits. Samples were transferred into 6 ml polymeric vials and analysed in duplicate, measuring the phosphate concentrations in order to calculate total P.

3.2.3 Inductively coupled plasma mass spectrometer

Metals analysis (Al and Fe in DWTP sludge and wastewater) was performed by inductively coupled plasma mass spectrometry (ICP-MS) following acid digestion of the samples. The principles of ICP-MS analysis have been described in detail by Thomas [170] and Batsala et al. [171]. 2 ml of sample was mixed with 8 ml of 1 mol 1^{-1} HNO₃ and added to a digestion vial. A duplicate sample was also used as a control. Lids were secured and the digestion vials heated in a CEM MARS 5 Microwave oven for 30 minutes at 120 °C (200 kPa).

Sludge samples were diluted $100 \times$, $500 \times$ and $1000 \times$, while wastewater samples were diluted $100 \times$. Standards of 5 mg/l, 1mg/l, 0.1 mg/l, 0.01 mg/l and 0.001 mg/l were prepared with multi-element ICP-MS standard (23 elements) before the analysis was completed by Jesper Knutsson, Doctor Qudent at Chalmers University of Technology.

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Parameter	Preparation/ Analysis	Method, Comments	Instrument(s)	Measurement range	Detection limit	Replication	Sample volume (ml)
	Preparation	Adapted from 3030 E – Standard Methods Nitric acid (HNO_3) digestion	CEM MARS 5 Microwave oven	-	-	-	-
Metals	Analysis	SS-EN ISO 17294-1:2006, SS-EN ISO 17294- 2:2005 Digested sludge diluted 1000x, 500x, 100x prior to analysis	Inductively coupled plasma mass spectrometer (ICP-MS)*	0 to 0.1 μg/l Al 0 to 0.1 μg/l Fe	0.001 μg/l A1 0.002 μg/l Fe	Triplicate	10
Alkalinity (as CaCO ₃)	Analysis	2320 B - Standard Methods Determination of carbonate alkalinity by titration method	Titration vessel	0 to 200 mg/l CaCO ₃	0.5 mg/l CaCO ₃	Single	50
рН	Analysis	Determination of pH using a pH-meter ISO 10523:1994	WTW Multi 350i pH/ISE/DO/conductivity measuring instrument	- 2.00 to 20.00	0.01	Single	50
	Preparation	Dilution and/or filtration prior to analysis	-	-	-	-	-
Organics (NOM as TOC and DOC)	Analysis	Determination of total organic carbon SS-EN 1484	Shimadzu TOC-VCPH Total Organic Carbon Analyser	0 to 25000 mg/l TC 0 to 30000 mg/l IC	4 μg/l	Duplicate	30
Total P	Preparation	Digestion with peroxodisulphate/alkali (Oxisolv oxidizing decomposition reagent) and filtration through 0.45 µm filter paper Adapted from 4500-P B.5 – Standard Methods**	CEM MARS 5 Microwave oven	-	-	-	-
	Analysis (Ion chromatography)	Determination of dissolved anions by liquid ion chromatography of ions SS-EN ISO 10304-2	Dionex ICS-900	0 to 500 μS	0.0001 μS	Duplicate	6
	Analysis (Colorimetric method)	Determination of reactive phosphorus with PhosVer 3 phosphate reagent (ascorbic acid method) 4500-P E – Standard Methods	Hach DR/890 Portable Colorimeter	420, 520, 560, 610 nm	1 nm	Duplicate	10
	Preparation	Dilution and/or filtration prior to analysis	-	-		-	-
Total N	Analysis	Pyrolysis and chemiluminescence detection for total nitrogen in water ASTM D5176 - 08	Shimadzu TN (Total Nitrogen) TNM-1 unit	0 to 4000 mg/l	5 µg/l	Duplicate	30

	Preparation	Vacuum filtration through 0.45 µm filter paper	Filtration apparatus (filter flask, aspirator and	-	-	-	-
solids	Analysis	Total suspended solids dried at 103–105 °C 2540 D – Standard Methods	Thermo Scientific drying oven, Sartorius	0 to 220g	0.1 mg	Duplicate	50
Total solids	Analysis	Total solids dried at 103-105 °C 2540 B – Standard Methods	Thermo Scientific drying oven, Sartorius analytical balance	0 to 220g	0.1 mg	Duplicate	50
	Analysis (water content)	Determination of dry matter and water content on a mass basis*** ISO 11465:1993	Thermo Scientific drying oven, Sartorius analytical balance	0 to 220g	0.1 mg	Duplicate	50
Demotorshilitor	Analysis (CST)	Tests on sludges – capillary suction time 2710 G – Standard Methods,	Triton Type 304M Laboratory CST	> 0.1 sec	0.1 sec	Duplicate	10
Dewaterability	Analysis (filtration)	Adaptation of time-to-filter test with vacuum filtration through 1.2 μm filter paper Adapted from 2710 H – Standard Methods	Filtration apparatus (filter flask, aspirator and filter funnel)	0 to 220g	0.1 mg	Duplicate	30
	Analysis (centrifugation)	Extraction of sludge pore water Adapted from 8080 C.1 – Standard Methods,	Sigma 4 -16 centrifuge, Sartorius analytical balance	0 to 220g	0.1 mg	Triplicate	50
Zone settling velocity	Analysis	Zone settling rate test with activated sludge Adapted from 2710 E – Standard Methods	11 measuring cylinders	0 to 1000 ml	2.5 ml	Duplicate	1000
Sludge volume index (SVI)	Analysis	Index of sludge ability to settle and compact Adapted from 2710 C, 2710 D and 2540 D – Standard Methods	11 measuring cylinders	Expected between 50 to 150 ml/g	-	Duplicate	5

*Sample preparation by the author, ICP-MS analysis carried out by Jesper Knuttson, Chalmers University of Technology.

**Method 'Determination of Total Phosphorus in Wastewater at Rya WWTP Laboratory' by Peter Robinsson, Gryaab AB.

***% solids calculated as dry matter expressed as a percentage of mass as per ISO 11465:1993 and equation 3.1 below.

% solids =
$$\frac{mass of total solids}{mass of sludge sample} \times 100$$
 (Eqn 3.1)

N.B. 'Standard Methods' are Standard Methods for the Examination of Water and Wastewater, 20th ed., 1999 [172]. Several dewaterability tests were carried out to determine classification of water distribution in sludge.

Table 3.1 Summary of analysis methodology for all analysed paramete

3.3 DWTP sludge characterisation

Characterisation of the alum DWTP sludges involved measurement of solid content (% solids), metals (Al and Fe), total N, organics (NOM (natural organic matter) as TOC (total organic carbon)) and pH by the methodology in Table 3.1.

Sludges were analysed 'as collected' for all analyses bar organics and total N analyses. To avoid the gelatinous metal hydroxide content and the aforementioned affinity for water of the sludge causing damage to/clogging the TOC analyser or total N unit, the sludges were oven dried, ground to a fine powder with a mortar and pestle and 50 mg dried sludge was re-suspended in 200 ml ultrapure (milli-Q) water.

Heavy metals (Zn, Cd, Pb), calcium (as CaO) and total P in Alelyckan DWTP sludge had been analysed by Nordén [13] in 2009 where samples were collected similarly from the bottom of one of four sedimentation tanks at Alelyckan DWTP. Data from Nordén [13] was included in the characterisation and these parameters were not re-analysed.

3.4 Wastewater characterisation

The wastewater posed no experimental handling challenges and all analyses were carried out to standard methods detailed in Table 3.1.

The first wastewater sample (collected 18th March 2014) was characterised in detail, with total solids, suspended solids, organics (NOM (natural organic matter) as TOC (total organic carbon) and DOC (dissolved organic carbon)), total N, total P, o-P, metals (Al and Fe), alkalinity (as CaCO₃) and pH measured to enable a classification of the wastewater by major chemical composition from Sincero & Sincero [138]. The first wastewater sample was used in the initial jar test (section 3.5).

The second wastewater sample (collected 16th May 2014) was characterised solely for comparison in the second jar test, with a more simple classification of organics (NOM (natural organic matter) as TOC (total organic carbon) and DOC (dissolved organic carbon)), total N, total P and o-P.

In both characterisations, the 'dissolved' organic carbon refers to compounds filterable through a 0.45 µm filter, before measurement with a Shimadzu TOC-VCPH TOC Analyser. Likewise, o-P was defined as the dissolved filterable 'reactive phosphorus' by Standard Methods 4500-P [172], measured via a colorimetric method with a Hach

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DR/890 Portable Colorimeter following reaction with the PhosVer 3 phosphate reagent (ascorbic acid method) as detailed in Table 3.1.

3.5 Jar testing

Two jar test experiments were carried out during this study. The first jar test, as detailed in Table 3.2 below, was run on the morning of 2nd April 2014 to determine effects of DWTP sludge on total P (measured by ion chromatography), pH and solids (settling) through analysis of total and suspended solids.

Following inconclusive results surrounding P removal (measured by ion chromatography) from the first jar test, a second jar test as detailed in Table 3.3 below, was run on the morning of 20th May 2014, analysing effects of DWTP sludge on total P (measured by a colorimetric method), total N and organics (TOC and DOC).

Jar test setup	Jar 1	Jar 2	Jar 3	Jar 4	Jar 5
DWTP sludge	None (control)	AL (current scenario)*	AL	LA-1	LA-5
Molar ratio (Al:PO ₄ - ³)	-	0.79:1*	3:1	3:1	3:1
Wastewater (l)	0.9	0.9	0.9	0.9	0.9
DWTP sludge added (ml)	0	2.06	7.81	10.25	23.32
Al added (mg Al/l	0	1.78	6.74	6.74	6.74
wastewater)					

*Actual molar ratio (Al: PO_4^{-3}) of current scenario defined following characterisation of first wastewater sample and definition of o-P/total P fraction

Jar test setup	Jar 1	Jar 2	Jar 3	Jar 4	Jar 5	Jar 6
DWTP sludge	None (control)	AL (current scenario)	AL	AL	AL (diluted)*	LA-5
Molar ratio (Al:PO ₄ ⁻³)	-	0.79:1	2:1	3:1	3:1	3:1
Wastewater (l)	0.9	0.9	0.9	0.9	0.9	0.9
DWTP sludge added (ml)	0	1.82	4.60	6.90	20.61	20.61
Al added (mg Al/l	0	1.57	3.97	5.96	5.96	5.96
wastewater)						

Table 3.2 Jar test 1 setup and DWTP sludge addition

*Dilution of AL sludge to model LA-5 sludge by dilution based on aluminium concentration. AL sludge diluted with ultrapure (milli-Q) water and refrigerated at 4 °C for 1 week prior to jar test.

Table 3.3 Jar test 2 setup and DWTP sludge addition

Jar tests were carried out with a Kemira Kemwater Flocculator with capacity for 6 parallel agitators in 1 l glass beakers controlled by predefined 'fast' (rapid mixing), 'slow' (slow mixing) and 'sediment' (quiescent settling) periods, shown during the first jar test in Fig. 3.1.

With all jars at room temperature with no pH control, experimentation was based on the following procedure with 900 ml wastewater samples each subject to the same agitation:

- 1) Addition of the desired volume and type of alum DWTP sludge by pipette
- 2) Rapid mixing (200 rpm) for 1 minute, slow mixing (40 rpm) for 30 minutes followed by a quiescent settling period for 60 minutes
- Careful pipetting of the required volume of sample for each analysis (in duplicate) (as per Table 3.1) from the surface of the settled wastewater (supernatant)



Fig. 3.1 Jar test 1 setup during slow mixing phase

3.6 Further solids (settling) testing

Investigation of alum DWTP sludge effects on secondary settling with activated sludge was carried out on 8th May 2014 in an attempt to clarify the limited conclusions on primary settling from literature and from the initial jar test.

3.6.1 Zone settling velocity and sludge volume index (SVI)

The high concentrations of suspended solids in activated sludge enable a clear zonesettling regime of settling under quiescent conditions with a distinct interface between the supernatant liquor and the sludge zone [172]. The height of the sludge interface was measured over a 30 minute period as 1 l of activated sludge settled under quiescent conditions at room temperature (20 °C). Tests were completed in duplicate in two 1 l glass measuring cylinders with 70 mm diameters. Plotting the curve of interface level against time enabled the zone settling velocity (m/hr) to be defined as the gradient of the linear part of the settling curve. The 30 min settlability (ml) was also recorded with increasing DWTP sludge addition, with the setup (and results) detailed in Table A.7 in Appendix A.

The sludge volume index (SVI), defined as the volume in milliliters occupied by 1 g of a suspension after 30 min settling [172], was also defined as per equation 3.2 below by Standard Methods 2710 D to characterise the activated sludge sample.

$$SVI = \frac{\text{settled sludge volume (ml/l)} \times 1000}{\text{suspended solids (mg/l)}}$$
(Eqn 3.2)

3.7 DWTP sludge handling

Experimentation evaluated the dewaterability of the DWTP sludge, including capillary suction time (CST) and classification of water distribution in the sludge.

3.7.1 Capillary suction time (CST)

The capillary suction time (CST) test evaluates the rate of water release from sludge via measurement of the time (in seconds) of liquid extraction from a sludge sample between two contact points on chromatography paper, in Standard Methods 2710 G [172]. Although typically used for sewage sludge, a short CST would indicate a filterable sludge while a longer CST would suggest slow and problematic filtration of DWTP sludge. The CST test was run using a Triton Type 304M Laboratory CST apparatus with a control of ultrapure (milli-Q) water and three DWTP sludges 'as collected' (AL, LA-1 and LA-5).

3.7.2 Water and solids content

As detailed in Table 3.1, a basic classification of water distribution in sludge from Veselind & Martel [102] was applied to alum DWTP sludge via a series of simple experiments. Free water was identified as the mass of water filterable through 0.45 μ m filter paper after 5 minutes using filtration apparatus. The mass of free and interstitial water was determined as the supernatant or centrate following centrifugation of the sludge at 4000 rpm for 5 minutes. The surface water and bound water could then be determined by subtracting the free and interstitial water from the total water content; the complement of the '% solids' from characterisation, and detailed in equation 3.1 earlier.

4. RESULTS

Results are presented in chronological order according to Methods and Materials, with characterisation of DWTP sludges (section 4.1) and wastewater (section 4.2) followed by results from jar testing and further solids (settling) with activated sludge (section 4.3) and DWTP sludge handling (section 4.4). Raw data is included Appendix A.

4.1 DWTP sludge characterisation

4.1.1 Initial experience of alum DWTP sludges

Experimentation with alum DWTP sludges was a new experience, with a uniform light brown colour and a feathery, bulky, gelatinous nature. The DWTP sludge settled well in storage, but as described in section 4.4, did not dewater easily. Despite apparently high water content, the sludges behaved as a gel, with Alelyckan sludge (AL) a 'thicker' gel than Lackarebäck sludges (LA-1, LA-5).

4.2.2 Characterisation and comparison

Table 4.1 on page _____ details the characterisation of DWTP sludges from this study along with averages from literature, taken from samples 2, 3, 5, 8, 9 and 10 in Table 2.8, where DWTP sludge samples had no treatment (dewatering or drying) during experimentation, although treatment prior to collection was likely. Comparison revealed a substantial dilution effect as a result of analysing the sludge without treatment, with all parameters very low compared to literature. More comparable data from a 2009 analysis of Alelyckan sludge [13], where treatment was likely, revealed a very high Zn concentration, a high Ca concentration, and low heavy metals concentrations.

Analysis of DWTP sludge 'as collected' without treatment (no dewatering or drying) was rare in literature, although a limited characterisation of alum DWTP sludge 'as collected' from Georgantas & Grigoropoulou [69] is detailed in Table 4.2 on page ____. Comparison suggested AL, LA-1 and LA-5 sludges were slightly weaker with lower metals concentrations for comparable solids %.

While proportionally the elemental constituents were somewhat comparable with some literature, with Al concentration dominating other constituents in the AL and LA-1 sludges and NOM (as TOC) proportionally high in the LA-5 sludge, it was clear that, as explicated in section 5.3, the composition and relative abundance of constituents were specific for each DWTP sludge.

				DWTP sludge				Comparison with previous studies
Parameter		Unit*	Date of analysis	AL	LA-1	LA-5	Average from literature	Comments, relative to previous studies
Solids	% solids	%	28-Mar	2.63	1.63	0.76	45.20	AL, LA-1, LA-5 very low. AL highest % solids.
	Al as Al_2O_3	mg/g	28-Mar	0.78	0.57	0.26	166.83	AL, LA-1, LA-5 very low. AL highest g/kg wet sludge. LA-5 lowest and .
	Fe as Fe_2O_3	mg/g	28-Mar	0.06	0.02	0.01	31.60	AL, LA-1, LA-5 very low.
Metals	Ca as CaO	mg/g	-	56	-	-	28.35	AL high.
	Zn	mg/kg	-	79	-	-	22.39	AL very high.
	Cd	mg/kg	-	< 0.30	-	-	0.50	AL low.
	Pb	mg/kg	-	6	-	-	15.32	AL low.
Р	Total P	g/kg	-	0.58	-	-	3.52	AL very low.
Ν	Total N	g/kg	25-Mar	0.02	0.01	0.01	-	Rarely analysed.
NOM	ТОС	g/kg	25-Mar	0.47	0.24	0.26	233.50	AL, LA-1, LA-5 very low. AL notably highest g/kg wet sludge, LA-5 notably highest relative to Al concentration.
pH		pН	26-Mar	7.71	7.72	7.69	6.68	AL, LA-1, LA-5 slightly alkaline.

- not analysed

*/kg or /g is per kg or g of wet DWTP sludge

Shaded data are from 2009 analysis of Alelyckan DWTP sludge by Nordén [173], where samples were collected similarly from the bottom of one of four sedimentation tanks at Alelyckan DWTP, although treatment (dewatering or drying) during experimentation was likely.

Table 4.1 Major chemical composition of Alelyckan and Lackarebäck DWTP sludge and comparison with literature

Parameter		Unit	Average from Georgantas & Grigoropoulou [69]
Solids	% solids	%	2.35
Matala	Al as Al_2O_3	mg/g	3.60
Metals	Fe as Fe_2O_3	mg/g	0.49

 Table 4.2 Chemical composition of alum DWTP sludge from Georgantas & Grigoropoulou [69]

4.2 Wastewater characterisation

Characterisations of the first and second wastewater samples from the Tankgatan sampling station and their classification according to Sincero & Sincero [138] from Table 2.10 are detailed in Table 4.3 and Table 4.4 respectively.

Parameter	Unit	Date of analysis	Concentration	Classification by Sincero & Sincero [138]
Total solids	mg/l	24-Mar	626.67	Weak-Medium
Suspended solids	mg/l	20-Mar	169.00	Weak-Medium
NOM (as TOC)	mg/l	19-Mar	116.60	Weak-Medium
NOM (as DOC)	mg/l	19-Mar	91.36	-
Total N	mg/l	19-Mar	18.77	Weak
Total P	mg P/l	26-Mar	3.73	Weak
o-P*	mg P/l	19-Mar	2.58	-
o-P/total P fraction	-	-	0.69	-
Al as Al_2O_3	mg/l	28-Mar	-1.34**	-
Fe as Fe_2O_3	mg/l	28-Mar	-0.86**	-
Alkalinity (as $CaCO_3$)	mg/l	26-Mar	114.00	Medium-Strong
pH	pH	26-Mar	6.43	-

- not analysed

Table 4.3 Chemical composition of wastewater (18-Mar-2014) from Tankgatan sampling station

Parameter	Unit	Date of analysis	Concentration	Classification by Sincero & Sincero [138]
NOM (as TOC)	mg/l	26-May	54.17	Weak
NOM (as DOC)	mg/l	26-May	20.89	-
Total N	mg/l	26-May	19.38	Weak
Total P	mg P/l	21-May	3.28	Weak
o-P*	mg P/l	20-May	2.28	-
o-P/total P fraction	-	-	0.70	-

- not analysed

Table 4.4 Chemical composition of wastewater (16-May-2014) from Tankgatan sampling station

The first wastewater sample was classified as a typical weak-medium wastewater with a distinctly high alkalinity (and hence a large 'buffering capacity' to neutralise acids and prevent significant quick changes in pH). The second wastewater sample was classified as a weak wastewater, likely more diluted than the first sample, although the o-P/total P fraction remained similar. Both samples were comparable in total P, total N and suspended solids to the earlier characterisations of municipal wastewater from Tankgatan sampling station in Table 2.9, although NOM (as TOC) was distinctly higher in the first sample.

The metals concentrations in Table 4.3 were detected as negative during ICP-MS analysis, although when compared to the magnitude of metals concentrations detected in the DWTP sludge (as shown in Table A.2 in Appendix A), and noting the potential for contamination of standard solutions, a comparably negligible concentration of metals in the wastewater would be detected as negative, as discussed later (section 5.3).

While data for total P and o-P in Table 4.3 and Table 4.4 were measured via a colorimetric method, Fig. A.1 and Table A.1 in Appendix A from ion chromatography highlighted the high concentrations of sulphate produced with thermal acidic persulphate digestion and its potential to interfere with measuring phosphate concentrations, as discussed in section 5.4. Sulphate represented 98.52% of the relative area in the chromatogram while the phosphate peak was barely visible in Fig. A.1.

4.3 DWTP sludge effects on primary wastewater treatment

4.3.1 Chemical analysis

Alum DWTP sludge effects on P removal, organics, total N, pH, primary settling (total and suspended solids) from jar testing, and secondary settling with activated sludge are described. In both jar tests, AL sludge (the dominant sludge by total solids received at the Rya WWTP) was dosed in increasing concentrations while LA-1 and LA-5 sludges were dosed only at the artificially high molar ratio (Al:PO₄⁻³) of 3:1. Raw data from jar testing is detailed in Table A.3 and Table A.4 in Appendix A.

4.3.1.1 P removal

Fig. 4.1 shows increasing addition of AL DWTP sludge resulted in increased removal of total P, with removal enhanced by almost 30% at the molar ratio (Al:PO₄⁻³) of 3:1. However, the model of the current scenario at the molar ratio (Al:PO₄⁻³) of 0.79:1 enhanced removal by only 4% from settling alone. Enhancement of total P removal was comparable with the practically relevant study from Georgantas & Grigoropoulou [69] as shown in Fig. 4.2 below, although total P removal was less efficient at lower doses of DWTP sludge.

Total P data was from jar test 2, with total P measured by colorimetric method following digestion and filtration. As discussed in section 5.7, total P data from jar test 1 was measured by ion chromatography and distorted by high sulphate concentrations. Consequently, no data from ion chromatography is included in this study.



Fig. 4.1 Effluent total P concentration with AL DWTP sludge addition



Fig. 4.2 Comparison of total P removal (AL DWTP sludge addition) with literature [69]

N.B. '0 molar ratio' represents control jar, with no alum DWTP sludge addition.

Comparison of P removal by alum DWTP sludge and alum

Using the molar ratio for alum (Al:total P), based on case histories from Guyer [83] and others [18] [35] with a molar ratio (Al:o-P) of 2:1, and adjusted for the o-P/total P fraction for wastewater from Tankgatan sampling station, the alum DWTP sludge in this study was estimated at between 3.5 and 10 times less effective than alum for total P removal (as Al) depending on dose and type of sludge. LA-5 sludge was more efficient (as Al) than AL sludge at the same high dose (molar ratio (Al:PO₄⁻³) 3:1).

DWTP sludge type and molar ratio (Al:PO ₄ ⁻³)	AL sludge (0.79:1)	AL sludge (3:1)	LA-5 sludge (3:1)		
Total P removed (from control) (mg/l)	0.13	0.97	1.41		
Al added (as DWTP sludge) (mg Al/l)	1.57	5.96	5.96		
Molar ratio for DWTP sludges (Al:total P removed)	13.83:1	7.05:1	4.85:1		
Molar ratio for alum* (Al:total P)	1.40:1				
Dose of alum DWTP sludge required compared to alum	9.9 times greater	5 times greater	3.5 times greater		

Table 4.5 Comparison of alum DWTP sludge performance with alum

4.3.1.2 Organics, total N and pH

As shown in Fig. 4.3, total N showed negligible change (maximum 5%) relative to the control jar with increasing addition of AL sludge. DOC removal was enhanced slightly, up to 9% enhanced removal at the molar ratio (Al:PO₄⁻³) of 3:1. However, TOC in the supernatant was increased by almost 30% relative to the control at the molar ratio (Al:PO₄⁻³) of 0.79:1. Higher doses of AL sludge reduced TOC concentrations, although it remained 6% higher than without sludge addition at the molar ratio (Al:PO₄⁻³) 3:1.



N.B. Organics (TOC, DOC) and total N data from jar test 2, where '0 molar ratio' represents control jar.

Changes in pH, measured in jar test 1, were too small to be seen graphically and are detailed in Table 4.6 below. A maximum pH change from 6.54 to 6.59 was noted even at the high molar ratio (Al:PO₄⁻³) of 3:1.

DWTP sludge type and	Control, no	AL sludge	AL sludge	LA-1	LA-5
molar ratio (Al:PO ₄ ⁻³)	sludge	(0.79:1)	(3:1)	sludge (3:1)	sludge (3:1)
pH	6.54	6.55	6.58	6.58	6.59

 Table 4.6 pH variation and % change from control jar with DWTP sludge addition

4.3.1.3 Primary settling

Solids (settling) data from jar test 1 is summarised in Fig. 4.4 below. Without sludge addition, total and suspended solids removal due to settling was 32% and 88% respectively. At a low dose of AL sludge (molar ratio (Al:PO₄⁻³) 0.79:1), this was enhanced slightly by 7% and 4% respectively while at the higher dose of AL sludge (molar ratio (Al:PO₄⁻³) 3:1) the total and suspended solids removal was negligible, with a 3% increase in suspended solids and 4% decrease in total solids relative to the control jar. Conversely, at the higher doses of LA-1 and LA-5 sludges (molar ratio (Al:PO₄⁻³) 3:1) total and suspended solids removal was enhanced by up to 6%.



Fig. 4.4 Effluent suspended and total solids concentration with DWTP sludge addition

4.3.1.4 Secondary settling

As shown in Fig. 4.5 below, increased addition of AL sludge decreased the zone settling velocity and increased the 30 minute settlability of the activated sludge relative to the control. Additionally, as detailed in Table A.7 in Appendix A, when 10 ml of DWTP sludge was added, AL sludge decreased the zone settling velocity and increased the 30 minute settlability more than the LA-1 sludge.

Much like when high volumes of sludge were dosed in the jar test, as shown in jars 3, 4 and 5 of Fig. 3.1, the supernatant liquor was left turbid and light brown in colour after the sludge zone had settled when increasing volumes of AL sludge were added. The SVI of the activated sludge control, detailed in Table A.6 in Appendix A, was relatively low at 95.56 ml/g compared to the typical SVI in an activated sludge plant of 50 to 150 ml/g [174].



Fig. 4.5 Effect of AL DWTP sludge addition on settling of activated sludge

N.B. '0 molar ratio' represents control with no sludge addition.

4.3.1.5 DWTP sludge comparison in treatment efficiency

Evaluation of the efficiency of sludges at high doses (molar ratio (Al:PO₄⁻³) 3:1) revealed AL sludge was most efficient in removal of all parameters per g of sludge added. Dilution of AL sludge to model LA-5 sludge based on aluminium concentration revealed the LA-5 sludge was generally more efficient as aluminium despite the AL sludge being more efficient as sludge.



Fig. 4.6 Comparison of efficiency (% change from influent per g of DWTP sludge added) of AL and LA-5 sludges at molar ratio (Al:PO₄⁻³) 3:1



4.3.2 Economic analysis

An economic analysis of the effects of alum DWTP sludge summarised in Table 4.7 below was based on total P only, and based on data from the model of the current scenario, molar ratio (Al:PO₄⁻³) 0.79:1 with AL DWTP sludge, assuming DWTP sludge from Alelyckan and Lackarebäck share this similar P removal rate. The change from the control jar calculation excludes P removal due to settling, and reduces the potential savings of alum DWTP sludge in the current scenario by a factor of 1

Parameter	Change from influent wastewater	Change from control jar
Total P removal per g of DWTP sludge (mg)	0.56	0.065
DWTP sludge from Alelyckan* (tonnes/year)	33987	33987
DWTP sludge from Lackarebäck* (tonnes/year)	66590	66590
Total DWTP sludge received at Rya WWTP (tonnes/year)	100577	100577
Total P removal by DWTP sludge (tonnes/year)	56.01	6.51
Equivalent iron sulphate saved** (tonnes/year)	426.68	49.61
Equivalent cost of iron sulphate saved (SEK/year)***	238940.32 (≈ £21504.63)	27783.76 (≈ £2500.54)

*Quantities of sludge based on total solids from Alelyckan and Lackarebäck DWTP sludge received at Rya WWTP in 2012 (from Tumlin [6]) and characterisation of AL and LA-5 sludges in Table 4.1. **Based on calculated weight ratio for iron sulphate (Quickfloc S, 17.6% Fe²⁺) in Table 2.2. ***2013 pricing of Quickfloc S for Gryaab AB of SEK 560 per tonne [85].

Table 4.7 Economic analysis of alum DWTP sludge for P removal at Rya WWTP

4.4 DWTP sludge handling

4.4.1 Capillary suction time (CST)

Denometer	Unit	Date of	DWTP sludge			
rarameter	Umt	analysis	AL	LA-1	LA-5	
Dewaterability (as capillary suction time)	sec	20-Mar	3.7	_*	_*	_*

*no noticeable development of wetted area on blotting paper

Table 4.8 Dewaterability (as capillary suction time) analysis of DWTP sludges

The CST test was unsuccessful with all DWTP sludges demonstrated no noticeable liquid extraction onto the chromatography paper, even after several minutes of testing.

4.4.2 Water and solids content

The solids content and classification of water distribution in the DWTP sludges are detailed in Table A.5 and summarised in Fig. 4.7 below. While AL sludge has the lowest free water content, it was noted during experimentation that while filtration (to determine the free water content) was successful for the LA-1 and LA-5 sludges, the AL sludge rapidly clogged the filtration apparatus. Although experimentation was conducted in duplicate, an artificially low free water content may have been recorded.

Finally, although all sludges had a low solids content, the sludges had a negligible metals content in the supernatant when compared to the compared to the magnitude of metals concentrations detected in the sludge as detailed in Table A.2 in Appendix A, such that the majority of metals are found in the sludge solids.





5. DISCUSSION

5.1 Current understanding of alum DWTP sludge effects on P removal

5.2 Sample storage and collection

5.3 Characterisation and comparison of DWTP Sludge

- 5.3.1 Initial experience of DWTP sludge
- 5.3.2 Characterisation and comparison
- 5.4 Characterisation of wastewater

5.5 Effects of alum DWTP sludge on wastewater treatment at the Rya WWTP

5.5.1 Chemical effects

- 5.5.1.1 Influent characteristics
- **5.5.1.2** Anaerobic digestion and production of biogas
- 5.5.1.3 Heavy metals and REVAQ certification
- 5.5.1.4 P removal

5.5.1.5 Solids (settling)

Primary settling

Secondary settling

5.5.1.6 Organics, total N and pH

- 5.5.2 Comparison of sludges in treatment efficiency
- 5.5.3 Economic effects
- 5.5.4 Environmental effects

5.6 DWTP sludge handling

- 5.6.1 Capillary suction time (CST)
- 5.6.2 Water and solids content
- 5.6.3 DWTP sludge processing method

5.7 Experimental methodology

5.7.1 Analysis of total P

6. CONCLUSIONS

7. RECOMMENDATIONS AND SUGGESTIONS FOR FUTURE WORK

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APPENDICES

Appendix A – Raw data

A.1 Wastewater characterisation

Characterisation of digested and filtered wastewater sample by ion chromatography



Fig. A.1 IC chromatogram (anions) of digested and filtered wastewater sample

Peak name	Retention time (min)	Area (µS×min)	Relative area (%)	Amount (mM)
Chloride	4.714	0.557	0.48%	0.1654
Nitrate	7.847	0.962	0.82%	0.2833
Phosphate	9.364	0.214	0.18%	0.0945
Sulphate	12.754	115.387	98.52%	69.7272

Table A.1 IC integration results (anions) of digested and filtered wastewater sample

A.2 DWTP sludge characterisation

				Concentration					
Parameter		Unit	Date of analysis	DWTP sludge			DWTP sludge supernatant		
				AL	LA-1	LA-5	AL	LA-1	LA-5
Matala	Al as Al_2O_3	mg/l*	28-Mar	777.13	591.73	260.15	-7.74	-1.83	-1.57
wietais	Fe as Fe_2O_3	mg/l*	28-Mar	62.86	25.77	5.00	-6.42	-1.11	-0.99

* /l is per l of wet DWTP sludge without treatment (no dewatering or drying).

Table A.2 Metals analysis of DWTP sludge and DWTP sludge supernatant from centrifugation

A.3 Jar testing results

Devementar	TIm:4	Date of	Concentration					
Farameter	Omt	analysis	Jar 1	Jar 2	Jar 3	Jar 4	Jar 5	
DWTP sludge type and molar ratio (Al:PO ₄ ⁻³)			Control,	AL	AL	LA-1	LA-5	
	-	-	no	sludge	sludge	sludge	sludge	
			sludge	(0.79:1)	(3:1)	(3:1)	(3:1)	
Total solids	mg/l	03-Apr	428.00	384.00	440.00	416.00	388.00	
Suspended solids	mg/l	02-Apr	20.00	14.00	24.00	12.00	10.00	

Table A.3 Analysis of total and suspended solids from jar test 1
Demomentar	TIm:4	Date of	Concentration						
rarailleter	Umt	analysis	Jar 1	Jar 2	Jar 3	Jar 4	Jar 5	Jar 6	
DWTP sludge type and molar ratio (Al:PO ₄ ⁻³)	-	-	Control, no sludge	AL sludge (0.79:1)	AL sludge (2:1)	AL sludge (3:1)	AL sludge (diluted) (3:1)	LA-5 sludge (3:1)	
Total P*	mg/l	21-May	2.29	2.16	1.82	1.32	1.24	0.88	
TOC	mg/l	27-May	22.72	28.79	25.84	24.09	23.42	21.93	
DOC	mg/l	27-May	15.15	14.11	13.84	13.73	13.69	11.41	
Total N	mg/l	27-May	16.57	17.36	17.35	16.69	16.40	16.42	

* Total P measured by colorimetric method following digestion and filtration

Table A.4 Analysis of otal P, TOC, DOC and total N from jar test 2

A.3 DWTP sludge handling

Devementer		Date of	DWTP sludge		
rarameter	Umt	analysis	AL	LA-1	LA-5
Wet sludge sample	g/l	28-Mar	996.67	1074.00	995.33
Solids (oven drying)	g/l	28-Mar	26.24	16.92	7.59
Total water	g/l	28-Mar	970.43	1057.08	987.75
Solids content	%	28-Mar	2.63%	1.58%	0.76%
Total water content	%	28-Mar	97.37%	98.42%	99.24%
Free water (filtration)	%	28-Apr	21.70%	33.30%	38.30%
Interstitial and free water (centrifugation)	%	20-Mar	62.20%	74.00%	82.50%
Interstitial water*	%	-	40.50%	40.70%	44.20%
Surface and bound water*	%	-	35.20%	24.40%	16.70%

*Calculated as per Vesilind & Martel [102]

Table A.5 Solids and water content of AL, LA-1 and LA-5 sludges

A.4 Secondary settling

Suspended solids (mg/l)	2760.00
Sludge volume index (SVI) (ml/g)	95.56

Volume (and type) of DWTP sludge added (ml)	Al added as sludge (mg Al/l)	Date of analysis	Gradient* (cm/min)	Zone settling velocity(m/hr)	30-min settlablility (ml)
0 (control, no sludge)	0.00	09-May	4.86	2.92	263.75
10 (LA-1)	5.92	09-May	3.81	2.28	271.25
10 (AL)	7.77	09-May	3.49	2.10	287.50
20 (AL)	15.54	09-May	3.33	2.00	292.50
30 (AL)	23.31	09-May	2.84	1.71	301.25
40 (AL)	31.09	09-May	2.13	1.28	362.50
50 (AL)	38.86	09-May	1.95	1.17	387.50

*Gradient of linear part of settling curves defines the zone settling velocity

Table A.7 Addition of alum DWTP sludge to activated sludge in zone settling rate tests

Appendix B – Sludge and wastewater sample collection

B.1 Sample collection



Fig. B.1 Collection of sludge and wastewater samples. a) and b) Manual collection (by the author) of DWTP sludge sample from drained settling tank at Alelyckan DWTP, used with permission from Professor Britt-Marie Wilén, Chalmers University of Technology. c) Tap enabling collection of DWTP sludge sample from storage at Lackarebäck DWTP, taken by the author at the site. e) and f) Collection of activated sludge from secondary settling tanks at Rya WWTP, taken by the author at the site.



B.2 Map of Gothenburg and sample collection and analysis locations

Fig. B.2 Sampling/analysis locations. Map data © 2013 Nokia © 2014 Microsoft Corporation

Appendix C – Project Management

Project Name: Evaluating Reuse of Sludge from Water Treatment for Wastewater Treatment Kev Unscheduled action Early Delayed Milestone Preparation ~ Scheduled action Break First Half-Session Time (week beginning Monday) 25-Nov-13 30-Dec-13 14-Oct-13 -Dec-13 -Dec-13 -Dec-13 -Dec-13 04-Nov-13 11-Nov-13 13 28-Oct-13 20-Jan-14 23-Sep-13 07-Oct-13 21-Oct-13 06-Jan-14 18-Nov-13 13-Jan-14 27-Jan-1 30-Sep-1 Start End 02--60 16-23-Work Breakdown Structure Tasks 1 Preparation and planning 1.1 Contact Erasmus at Chalmers University 23-Sep 10-Oct 1.2 Contact supervisor at Chalmers University 23-Sep 18-Oct 1.3 Select and agree project specification 14-Oct 04-Nov ~ 1.4 Meet project supervisor* (University of Aberdeen) 14-Oct 21-Nov ~ 1.5 Complete application to Chalmers University 30-Sep 22-Nov 02-Dec 20-Dec 1.6 Complete Erasmus documents 2 Confirmation of project and background research 2.1 Background research 18-Nov 31-Jan 2.2 Contact project partners 02-Dec 20-Dec 2.3 Arrange accomodation, flights, insurance, etc. 09-Dec 20-Dec Christmas 2013 break 23-Dec 03-Jan 2.4 Visit to Gothenburg and Chalmers University 03-Jan 06-Jan January 2014 exams/interviews 13-Jan 24-Jan 2.5 Final travel to Gothenburg 27-Jan 01-Feb Second Half-Session Time (week beginning Monday) 03-Mar-14 12-May-14 l 7-Mar-14 4 14 14 14 14 05-May-14 l 9-May-14 26-May-14 09-Jun-14 4 $\frac{1}{4}$ l 0-Mar-14 02-Jun-14 16-Jun-14 23-Jun-14 4 14 24-Mar-1 14-Apr-1 21-Apr-1 31-Mar-1 07-Apr-1 28-Apr-24-Feb-30-Jun-Feb-Feb-Feb-Start End 03-0 5 Work Breakdown Structure Tasks 3 Introductory and familiarity stages 3.1 Meet project supervisor** (Chalmers University) 03-Feb 07-Feb 3.2 Meeting 1*** 03-Feb 07-Feb 03-Feb 14-Feb 3.3 Define overall aim and research objectives 3.4 Literature review 10-Feb 28-Feb 17-Feb 21-Mar 3.5 Familiarity with experimental techniques 4 Experiments and Analysis 4.1 Collection of wastewater and DWTP sludge 17-Mar 21-Mar 4.2 Characterisation of samples 17-Mar 28-Mar 4.3 Complete jar test 1 31-Mar 04-Apr 4.4 Analysis of jar test 1 31-Mar 11-Apr Easter break 14-Apr 18-Apr 4.5 Complete handling and settling tests 28-Apr 09-May 19-May 23-May 4.6 Complete jar test 2 4.7 Analysis of jar test 2 26-May 30-May 5 Drafting thesis and presentations 5.1 Preparation of presentation/meeting 2 05-May 09-May 5.2 Presentation/meeting 2 *** 05-May 09-May 5.3 Draft thesis 26-May 20-Jun 5.4 Preparation of presentation/meeting 3 09-Jun 13-Jun 6 Submission and presentations 6.1 Submission of draft thesis (Chalmers University) 16-Jun 18-Jun 6.2 Submission of thesis (Chalmers University) 30-Jun 01-Jul 6.3 Submission of thesis (University of Aberdeen) 30-Jun 01-Jul 6.4 Presentation (University of Aberdeen) 15-Sep 19-Sep

Project Management - Gantt Charts - First and Second Half-Sessions 2013-2014

*Supervisor meetings continued on a weekly basis (after lectures) at University of Aberdeen

**Supervisor meetings continued at least on a weekly basis at Chalmers University

***Meetings with Kretslopp och vatten, Göteborg Stad, Gryaab AB and Professor Britt-Marie Wilén, Chalmers University

Fig. C.1 Gantt charts with work breakdown structure for first and second half sessions (2013/2014) of project